

## 6.0 TRAFFIC IMPACT ASSESSMENT

### 6.1 Introduction

The proposed hotel developments would induce limited additional traffic to the surrounding road network. Nevertheless, in view of the nature of the developments, it is anticipated that the majority of these hotel trips would be tourism-based trips or leisure trips which would largely take place during off-peak hours on weekdays as well as on weekend and public holidays.

It is anticipated that only minimal additional traffic volume will be generated during the normal weekday morning and evening peak hours which are generally between 0800 to 0900 and 1730 to 1830. Nevertheless, traffic impact assessments were undertaken to identify potential traffic issues and the likely impact of the proposed three hotel developments on the surrounding road network.

### 6.2 Proposed Development Schedule

#### 6.2.1 Target Completion Year

According to the latest planning framework, the proposed development size and targeted completion year for the three proposed hotel developments are summarized in Table 6.1.

Ocean Hotel which is adjacent to the Entry Plaza of Ocean Park is scheduled for completion in line with Ocean Park Redevelopment Plan which commenced construction work in November 2006.

**Table 6.1 Target Completion Year of Proposed Hotels**

Name of Hotel	No. of Guest Rooms	Target Completion Year
Ocean Hotel	660	2010 - 2011
Fisherman's Wharf Hotel	460	2012 - 2013
Spa Hotel	180	2012 - 2013

#### 6.2.2 Vehicular Access Arrangement

The existing Ocean Park Road, via the Roundabout, will serve as the primary vehicular access for both Ocean Park and the Ocean Hotel. A new access road will branch off from Ocean Park Road which runs along the northern side of Ocean Park and Ocean Hotel. The new access road will further branch-off to provide separate accesses leading to the car parking area, loading/unloading area, taxi/private car drop-off lay-bys and the coach drop-off area of the Hotel.

Vehicular access to both the Fisherman's Wharf Hotel and Spa Hotel will be via the existing Shum Wan Road connecting from Nam Long Shan Road. The existing emergency vehicle access (EVA) at the Tai Shue Wan Entrance currently serving the Headland will be upgraded to serve as an EVA and private access to the Spa Hotel.

#### 6.2.3 Emergency Vehicular Access

EVA will be provided in each hotel along the major façade of the hotel building. The alignment of the EVA is designed to ensure the horizontal distance from the EVA to the major facade would not exceed 10m and that the part of the EVA serving the major facade is not covered.

For access to the Spa Hotel, the existing EVA connecting from Tai Shue Wan roundabout to Ocean Park Headland is a substandard private access road. Currently, it is also a service road used by goods and servicing vehicles.

This access road currently does not satisfy the minimum requirement for EVA as stipulated in the "Code of Practice for Means of Access for Firefighting and Rescue" published by Building Department of HKSAR. It is therefore proposed to upgrade the access road to minimum 6.0 m wide and the radius of curvature will comply with the standard of minimum 6.1m for inner turning radius and an outer turning radius of minimum 11.0 m. Also, the existing minimum headroom clearance of 4.5 m along the EVA will be maintained.

#### 6.2.4 Parking and Loading / Unloading Facilities

Parking, pick-up and drop-off lay-bys as well as loading and unloading areas will be provided within the hotel developments. The proposed parking spaces and loading/unloading bays to be provided in each hotel are given in Table 6.2 below.

Based on the design layout, Ocean Hotel and Ocean Park will be using a common parking area in which 23 parking spaces will be allocated to Ocean Hotel.

The provision of facilities for the Spa Hotel as indicated in Table 6.2 will be located within the hotel development site. Additional lay-bys and loading/unloading bays will be provided in the Tai Shue Wan area to meet HKPSG minimum requirements.

Visitors to the Spa Hotel arriving by private car or taxi will be dropped off at the hotel entrance. Private cars will be parked either within the hotel site or in the Tai Shue Wan parking area. Shuttle services will be provided to carry visitors to/from the Spa Hotel and Tai Shue Wan parking area.

**Table 6.2 Provision of Parking and Loading/unloading facilities**

	Ocean Hotel		Fisherman's Wharf Hotel		Spa Hotel	
	HKPSG	Proposed Provision	HKPSG	Proposed Provision	HKPSG	Proposed Provision
Car Parking (guest room)	7	23	5	20	2	7
Car Parking (Banquet, Conference Room)	4		8		2	
Car Parking (Food & Beverage)	5		7		3	
L/UL Bay for Goods Vehicles	3	3	2 - 5	5	1	1
Lay-bys for Taxi / Private Car	4	3	3	-*	2	-*
Lay-by for Single-deck Tour Bus	2	2	2	-*	1	-*

Note:

1. Parking provision primarily based on hotel type (a) "Main Urban Areas & New Town" in HKPSG.
2. Numbers indicated under HKPSG refers to the HKPSG minimum requirement.
3. \* Hotel pick-up/drop-off for taxi and private car are provided at the main entrance.

### 6.3 Transportation

#### 6.3.1 Existing Public Transport Services

Ocean Hotel is very conveniently located adjacent to the Wong Chuk Hang Road where a good mix of public transport services, including franchised buses, green minibus, red minibus and taxis, are available for hotel guests heading to different parts of Hong Kong.

Apart from taxis, public transport services, including bus route no. 629 and GMB route no.29A, are available on Shum Wan Road. Currently, shuttle bus service route no. 888 also provides additional transportation connection between Ocean Park main entrance and Tai Shue Wan entrance which is operating on an ad-hoc basis. As such, it is considered that the existing public transport services would have sufficient capacity to cater for the guests of the hotel developments.

#### 6.3.2 Access to Ocean Park

In view of the location and nature of the three hotels, it is assumed that hotel guests would be visiting Ocean Park during their stay. In order to enhance accessibility and convenience for guests from Fisherman's Wharf Hotel and Spa Hotel visiting Ocean Park, consideration should be given to the provision of an alternative access to Ocean Park Headland/Summit to provide a direct connection between these two hotels and the Park.

### 6.4 Traffic Forecasting

#### 6.4.1 Traffic Survey

Critical road links and junctions in the vicinity of the development sites which are likely to be affected by the proposed hotel developments were identified for traffic impact assessment and are summarized in Table 6.3 and Table 6.4. The locations of these junctions and road links are indicated in Figure 6.1.

**Table 6.3 Identified Critical Junctions**

	Junction Name	Type
J1	Wong Chuk Hang Road/ Nam Long Shan Road	Signal
J2	Heung Yip Road/ Nam Long Shan Road	Signal
J3	Heung Yip Road/ Ocean Park Road	Roundabout
J4	Ocean Park Road/ Hospital Access Road	Signal
J5	Police School Road / Ocean Park Road / Heung Yip Road	Priority
J6	Police School Road / Nam Long Shan Road	Priority
J7	Nam Long Shan Road / Shum Wan Road	Signal

**Table 6.4 Identified Critical Links**

	Road Name	Section	
		From	To
L1	Aberdeen Tunnel	Toll Plaza	Tunnel Portal
L2	Wong Chuk Hang Road	Nam Long Shan Road	Ap Lei Chau Bridge Road
L3	Wong Chuk Hang Road	Nam Fung Road	Ocean Park Interchange

Classified manual traffic surveys were conducted at the identified critical road links and junctions for both morning and evening peak periods in October 2006 in order to determine the existing traffic conditions in the Wong Chuk Hang and Nam Long Shan areas. The surveyed locations and the observed traffic flows in the morning and evening peak hours are shown in Figure 6.2.

#### 6.4.2 Background Traffic Forecast

In developing the background traffic forecast for the current study, reference was made to the "Supplementary TIA for Repositioning and Long Term Operation Plan of Ocean Park (June 2005)" (hereinafter referred to as the "Previous TIA Study") issued by Maunsell Consultants Asia Ltd.

The Previous TIA Study provides important information on the volume of additional trips generated/attracted as a result of the Ocean Park Redevelopment Plan. Nevertheless, it is important to note that the three hotel developments were also assumed to be part of the Redevelopment Plan in the Previous TIA Study.

In order to assess the impact from the hotel developments, the previously assumed trips generated by the hotel development were taken out and the result then formed the design year "Reference Case" without the hotels for the current study. The latest trip generation based on the updated hotel development parameters were derived and added to the "Reference Case" totals to produce the "With Hotel Developments" scenario.

For the purposes of the current study, it is assumed that the MTRC South Island Line (SIL) will not be in place when the hotels become operational in view of the fact that SIL is still at the discussion stage and not as yet a committed project by the government. As such, the traffic assessment is based on forecasts assuming no SIL, which represents the worst scenario for the future background traffic conditions in the study area.

#### 6.4.3 Traffic Forecast with Proposed Developments

The estimated trips generated from the proposed hotel developments were based on relevant trip rates from Transport Department's Data Record No. 439 as extracted in Table 6.5. The estimated numbers of trips generated by the three hotels are shown in Table 6.6. As shown, the three hotels altogether are estimated to generate a maximum of 79 pcus and 91 pcus per hour per direction during the weekday morning and evening peak hours respectively.

**Table 6.5 Adopted Hotel Trip Generation/Attraction Rates from DR439**

	AM Peak	PM Peak
Generation (pcus/hour/room)	0.060	0.070
Attraction (pcus/hour/room)	0.060	0.070

**Table 6.6 Estimated Trip Generation of Proposed Hotels**

	No. of Rooms	AM Peak		PM Peak	
		In	Out	In	Out
Ocean Hotel	660	40	40	46	46
Fisherman's Wharf Hotel	460	28	28	32	32
Spa Hotel	180	11	11	13	13
<b>Total</b>	<b>1,300</b>	<b>79</b>	<b>79</b>	<b>91</b>	<b>91</b>

Based on the location of the three hotels, it is anticipated that trips generated to/from Ocean Hotel will access via the roundabout on Ocean Park Road and then be distributed to the surrounding road network via the Ocean Park Road and Interchange.

Similarly, the trips generated to/from Fisherman's Wharf Hotel and Spa Hotel will utilise Shum Wan Road and the three signal junctions along Nam Long Shan Road for outbound trips, while inbound trips will be heading via Ocean Park Road and Police School Road.

According to the hotel design brief prepared by the hotel consultant for the current study, the hotels would achieve a stabilized occupancy level within 3 years.

## 6.5 Traffic Impact Assessment

### 6.5.1 Methodology

Junction capacity analysis was carried out for the existing year 2006, design years 2011, 2016 and 2022 for the critical junctions that are likely to be affected by the hotel developments. For assessment purposes, it is assumed that the three hotel developments will be in place by 2011. Capacity analysis was carried out in accordance with the procedures outlined in the Transport Planning and Design Manual (TPDM).

### 6.5.2 Junction and Link Assessment

The results of the road junctions and links capacity analysis for existing year 2006, design years 2011, 2016 and 2022 "Reference" and "With Hotel" cases are summarised in Table 6.7 to 6.12. The calculation sheets are attached in Appendix 6A.

**Table 6.7 Existing Year and Opening Year Junction Performance**

	Junction	Reserve Capacity (%) / Ratio of Flow to Capacity (RFC)					
		Existing Year 2006		Opening Year 2011			
				Reference	With Hotels	AM	PM
J1	Wong Chuk Hang Road/ Nam Long Shan Road (W14)	51%	45%	54%	70%	52%	67%
J2	Heung Yip Road/ Nam Long Shan Road (W15)	67%	41%	58%	97%	49%	84%
J3	Heung Yip Road/ Ocean Park Road Roundabout	0.40	0.38	0.28	0.61	0.30	0.65
J4	Ocean Park Road/ Hospital Access Road	192%	240%	252%	166%	241%	158%
J5	Heung Yip Road/ Police School Road	0.09	0.17	0.31	0.24	0.31	0.24
J6	Nam Long Shan Road/ Police School Road	0.25	0.08	0.03	0.14	0.03	0.14
J7	Nam Long Shan Road / Shum Wan Road (W17)	54%	-16%	62%	44%	50%	29%

**Table 6.8 Design Years 2016 and 2022 Junction Performance**

	Junction	Reserve Capacity (%) / Ratio of Flow to Capacity (RFC)					
		Design Year 2016			Design Year 2022		
		Reference		With Development	Reference		With Development
		AM	PM	AM	PM	AM	PM
J1	Wong Chuk Hang Road/ Nam Long Shan Road (W14)	56%	62%	53%	59%	49%	52%
J2	Heung Yip Road/ Nam Long Shan Road (W15)	55%	107%	52%	100%	45%	84%
J3	Heung Yip Road/ Ocean Park Road Roundabout	0.41	0.68	0.43	0.72	0.42	0.82
J4	Ocean Park Road/ Hospital Access Road	260%	37%	248%	33%	237%	19%
J5	Heung Yip Road/ Police School Road	0.29	0.25	0.29	0.25	0.31	0.28
J6	Nam Long Shan Road/ Police School Road	0.03	0.14	0.03	0.14	0.03	0.14
J7	Nam Long Shan Road / Shum Wan Road (W17)	63%	53%	51%	38%	52%	33%

**Table 6.9 Existing Year 2006 Road Links Performance**

	Link	From	To	Dir.	Capacity (pcus/hr)	2006 Flows		V/C	
						AM	PM	AM	PM
L1	Aberdeen Tunnel	Toll Plaza	Tunnel Portal	NB	3,000	2,791	2,648	<b>0.93</b>	0.88
				SB	3,300	2,935	2,472	0.89	0.75
L2	Wong Chuk Hang Road	Nam Long Shan Road	Ap Lei Chau Bridge	EB	4,500	2,969	2,547	0.66	0.57
				WB	4,500	2,748	2,586	0.61	0.57
L3	Wong Chuk Hang Road	Ocean Park Interchange	Nam Fung Road	EB	4,500	3,267	3,141	0.73	0.70
				WB	4,500	3,342	3,001	0.74	0.67

**Table 6.10 Design Year 2011 Road Links Performance**

	Link	Dir.	2011 Reference		V/C		2011 With Hotels		V/C	
			AM	PM	AM	PM	AM	PM	AM	PM
L1	Aberdeen Tunnel	NB	3,349	2,915	<b>1.12</b>	<b>0.97</b>	3,387	2,961	<b>1.13</b>	<b>0.99</b>
		SB	2,817	2,074	0.85	0.63	2,855	2,120	0.87	0.64
L2	Wong Chuk Hang Road	EB	3,044	2,090	0.68	0.46	3,060	2,108	0.68	0.47
		WB	2,632	2,729	0.58	0.61	2,648	2,747	0.59	0.61
L3	Wong Chuk Hang Road	EB	3,249	3,145	0.72	0.70	3,310	3,218	0.74	0.72
		WB	3,317	2,484	0.74	0.55	3,378	2,557	0.75	0.57

**Table 6.11 Design Year 2016 Road Links Performance**

	Link	Dir.	2016 Reference		V/C		2016 With Hotels		V/C	
			AM	PM	AM	PM	AM	PM	AM	PM
L1	Aberdeen Tunnel	NB	3,669	3,655	<b>1.22</b>	<b>1.22</b>	3,707	3,701	<b>1.24</b>	<b>1.23</b>
		SB	3,367	2,474	<b>1.02</b>	0.75	3,405	2,520	<b>1.03</b>	0.76
L2	Wong Chuk Hang Road	EB	3,094	2,400	0.69	0.53	3,110	2,418	0.69	0.54
		WB	3,202	2,939	0.71	0.65	3,218	2,957	0.72	0.66
L3	Wong Chuk Hang Road	EB	3,309	3,605	0.74	0.80	3,370	3,678	0.75	0.82
		WB	3,907	2,784	0.87	0.62	3,968	2,857	0.88	0.63

**Table 6.12 Design Year 2022 Road Links Performance**

	Link	Dir.	2022 Reference		V/C		2022 With Hotels		V/C	
			AM	PM	AM	PM	AM	PM	AM	PM
L1	Aberdeen Tunnel	NB	3,869	3,985	<b>1.29</b>	<b>1.33</b>	3,907	4,031	<b>1.30</b>	<b>1.34</b>
		SB	3,597	2,614	<b>1.09</b>	0.79	3,635	2,660	<b>1.10</b>	0.81
L2	Wong Chuk Hang Road	EB	3,274	2,520	0.73	0.56	3,290	2,538	0.73	0.56
		WB	3,482	3,119	0.77	0.69	3,498	3,137	0.78	0.70
L3	Wong Chuk Hang Road	EB	3,489	3,925	0.78	0.87	3,550	3,998	0.79	0.89
		WB	4,187	2,944	<b>0.93</b>	0.65	4,248	3,017	<b>0.94</b>	0.67

## 6.6 Discussion

Congestion and tail back problems from the Cross Harbour Tunnel are commonly observed during the morning and evening peak hours at Aberdeen Tunnel. Moreover, manually controlled tidal flow traffic management is often required during peak hours to regulate traffic flow.

Based on the latest population forecast contained in a report entitled "Projections of Population Distribution, 2006-2015" published on 27 October 2006 by the Planning Department, the population in Hong Kong South District is expected to grow from 277,500 in base year 2005 to 284,700 in 2010 and 287,400 in 2015. As such, the capacity problem of Aberdeen Tunnel will continue to get worse if additional capacity cannot be provided in the near future.

Implementation of the South Island Line (SIL) would likely help to relieve the heavy road traffic demand in the southern district as a result of the shift in the transport modes of commuters from road-based modes to rail-based mode.

## 6.7 Summary of Findings

The proposed three hotel developments altogether are estimated to generate a maximum of 79 pcus and 91 pcus per hour per direction during the weekday morning and evening peak hours respectively.

Parking and loading/unloading facilities will be provided in accordance to HKPSG minimum requirement.

Existing public transport facilities and services are considered adequate to cater for the guests of the hotel developments.

Vehicular access to both the Fisherman's Wharf Hotel and Spa Hotel will be via the existing Shum Wan Road connecting from Nam Long Shan Road. The existing EVA at the Tai Shue Wan Entrance currently serving the Headland will be upgraded to serve as an EVA and private access road to the Spa Hotel.

Altogether 7 critical junctions and 3 major road links were identified for capacity assessment. The assessment was based on traffic forecasts assuming no South Island Line (SIL) which represents the worst scenario for the future background traffic conditions in the study area.

All the assessed junctions are found to operate within capacity in the three design years 2011, 2016 and 2022 in both the "Reference" scenario and "With Hotel" scenario where the proposed hotel developments are assumed in place. As such, the proposed hotel developments would not have significant impact to the surrounding road junctions.

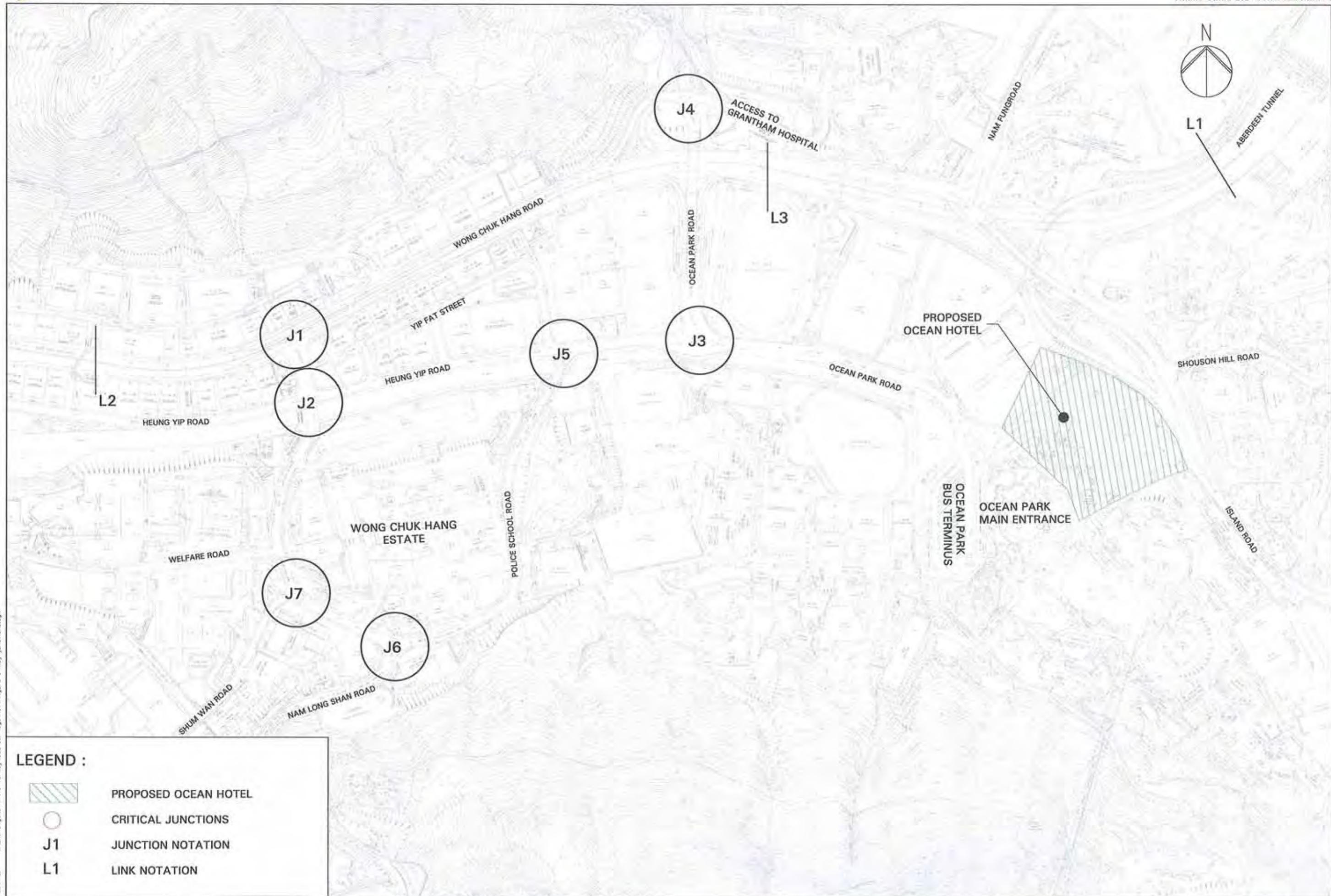
As for the assessed road links, it is found that Aberdeen Tunnel will be operating beyond capacity in both the "Reference Case" and "With Hotel Developments" scenarios. However, the small amount of additional trips from the proposed hotel developments would only affect the performance of the Tunnel very slightly by 1% in both design years 2011 and 2022. Therefore, it is envisaged that the hotel developments would not generate significant traffic impact to the Aberdeen Tunnel during peak hours.

It is considered that the South Island Line (SIL) which is currently under consideration and discussion by the HKSAR government, MTRCL and various parties would likely help to improve the traffic condition of Aberdeen Tunnel and the surrounding area.

## 6.8 Conclusions

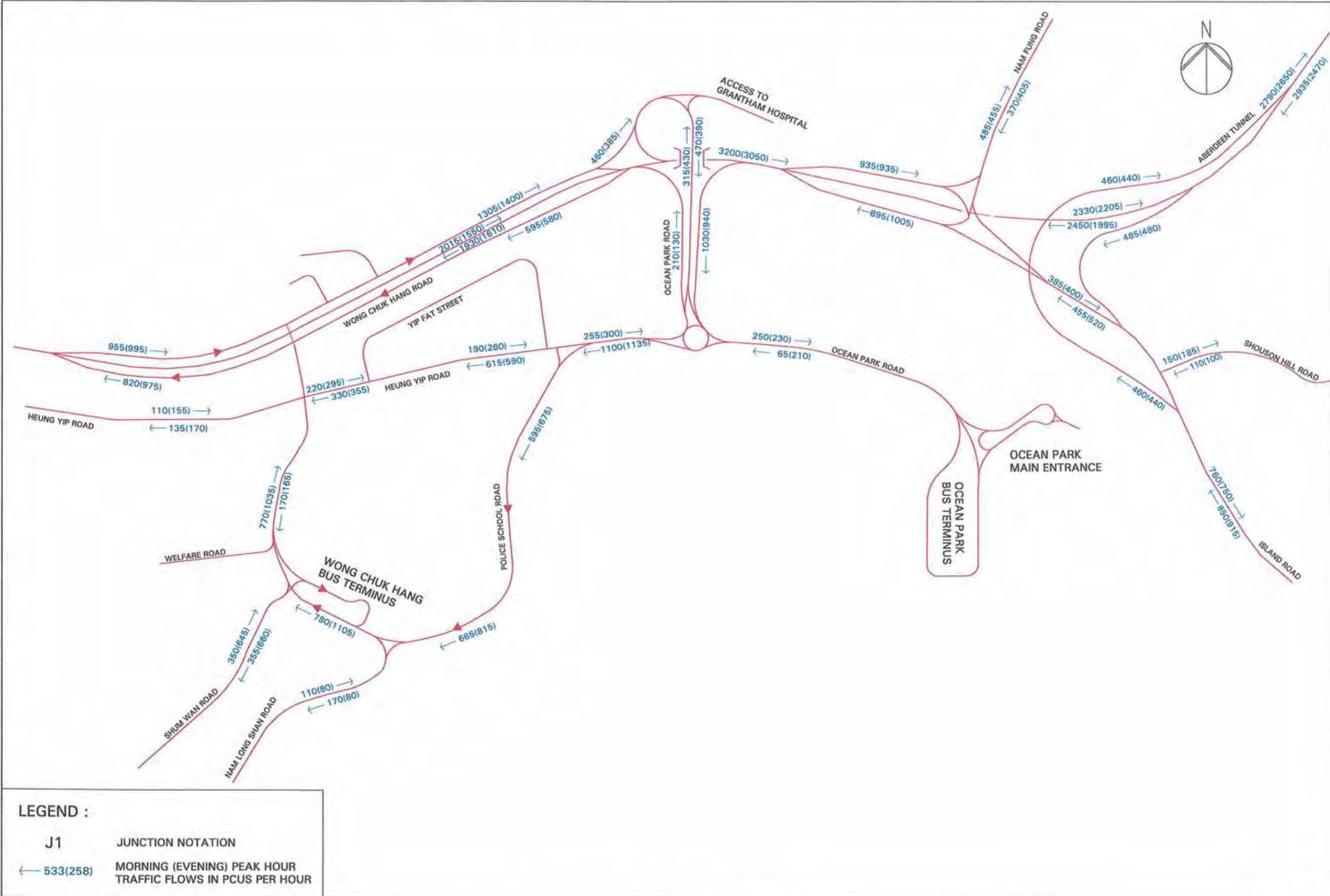
It is therefore concluded that the proposed hotel developments would not induce significant traffic impact and are therefore considered as feasible from a traffic point of view.

## Feasibility Study for Ocean Park Hotel Development Project



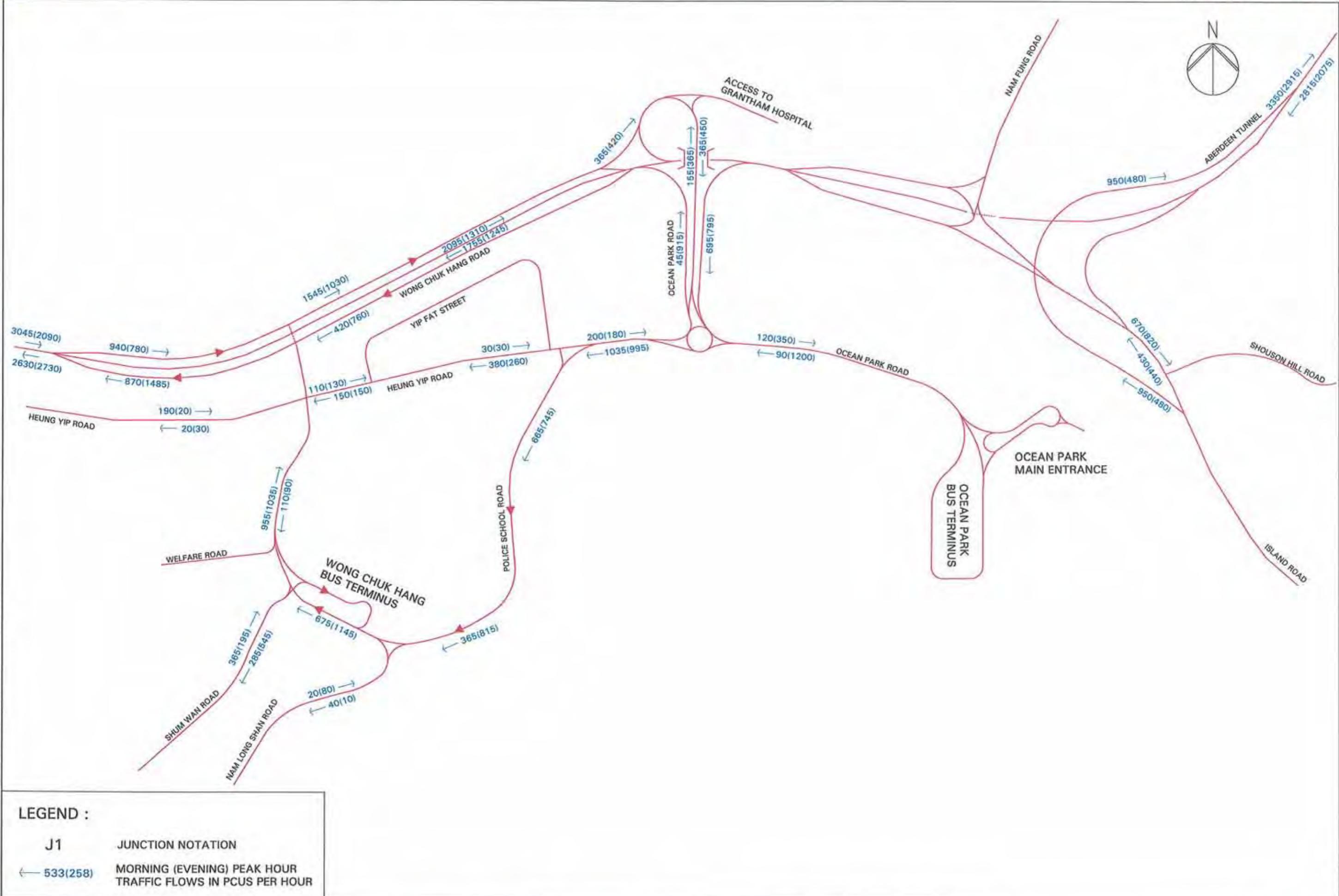
LOCATION OF CRITICAL JUNCTIONS AND MAJOR LINKS ASSESSED

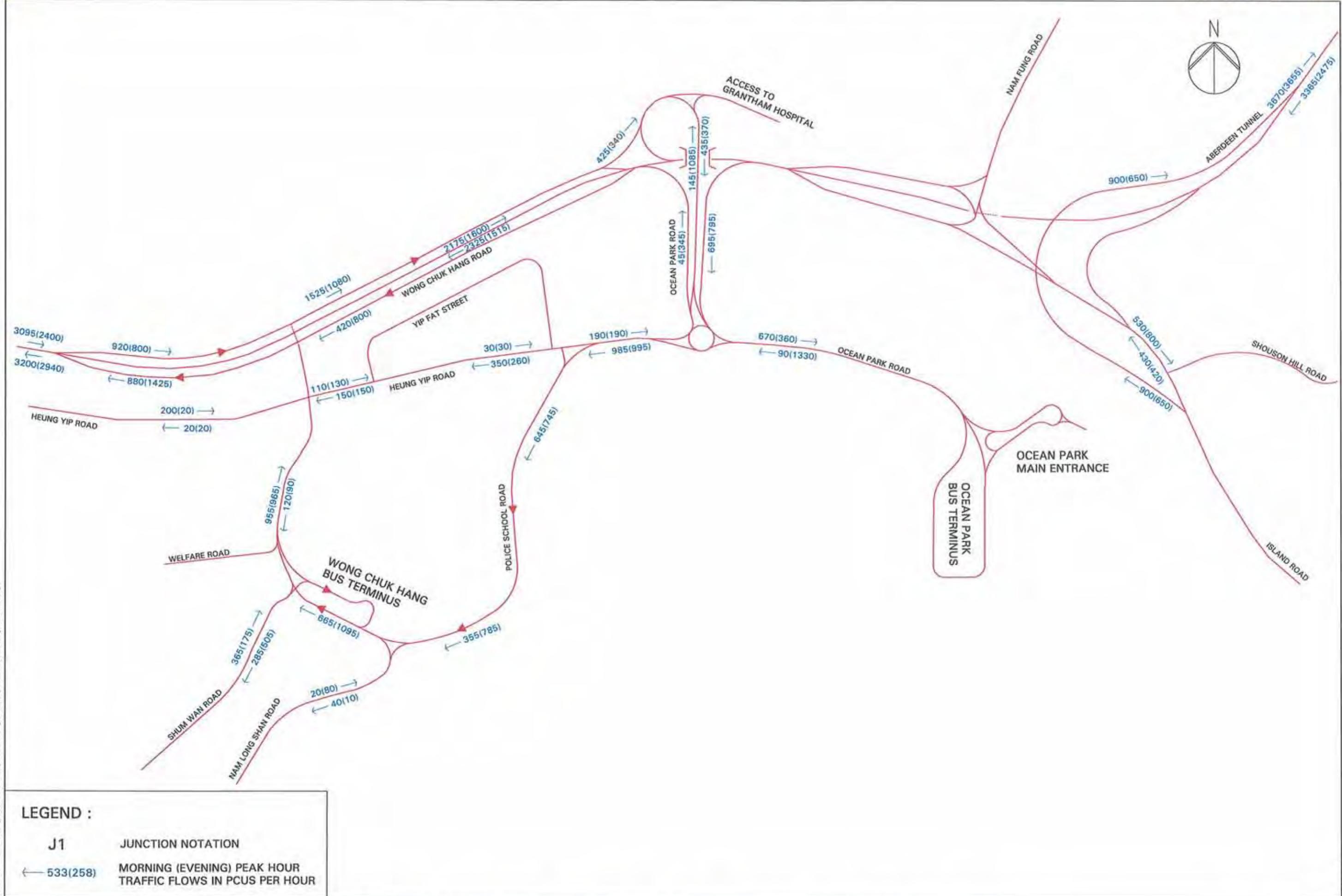
FIGURE 6.1

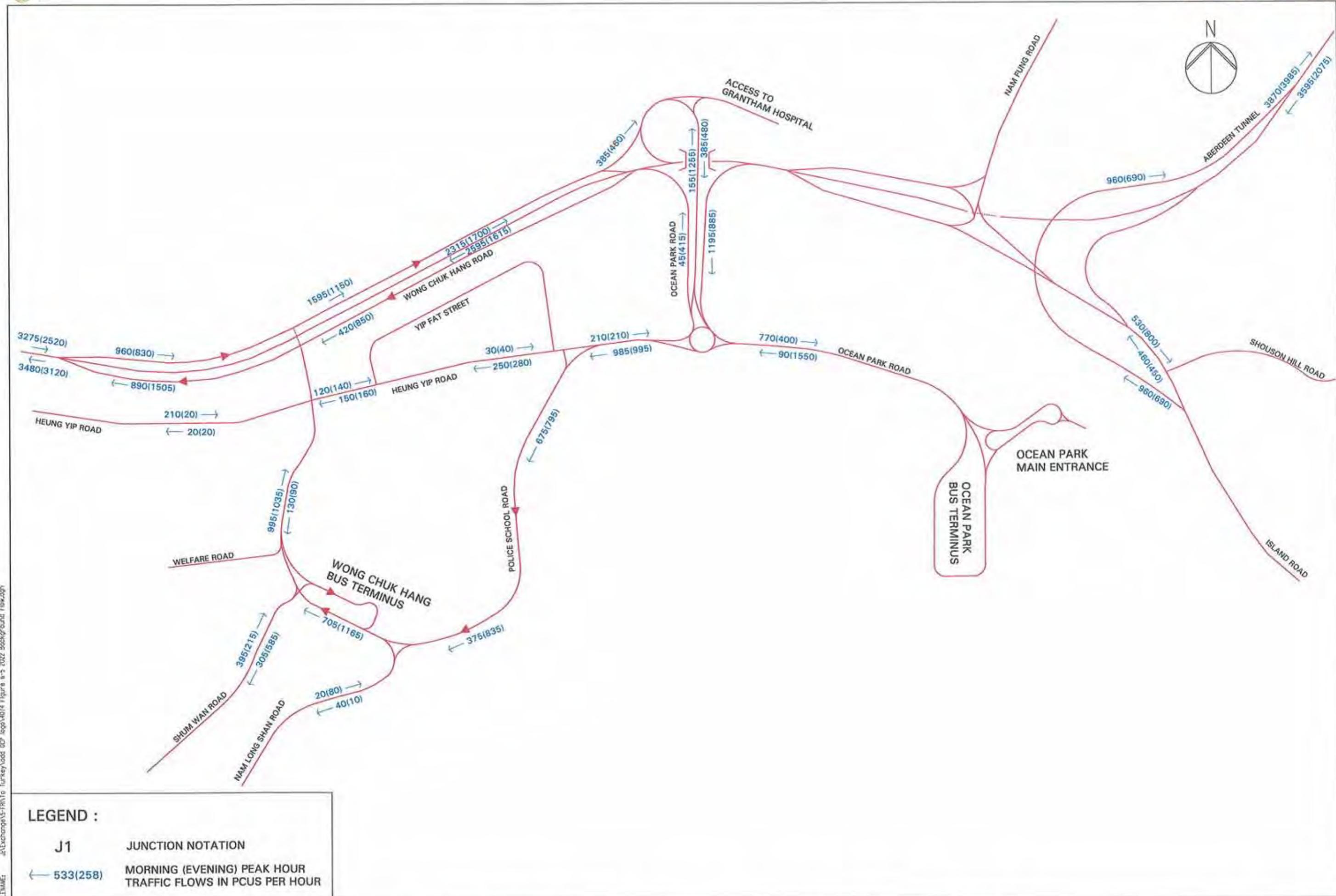


OBSERVED 2006 TRAFFIC FLOWS

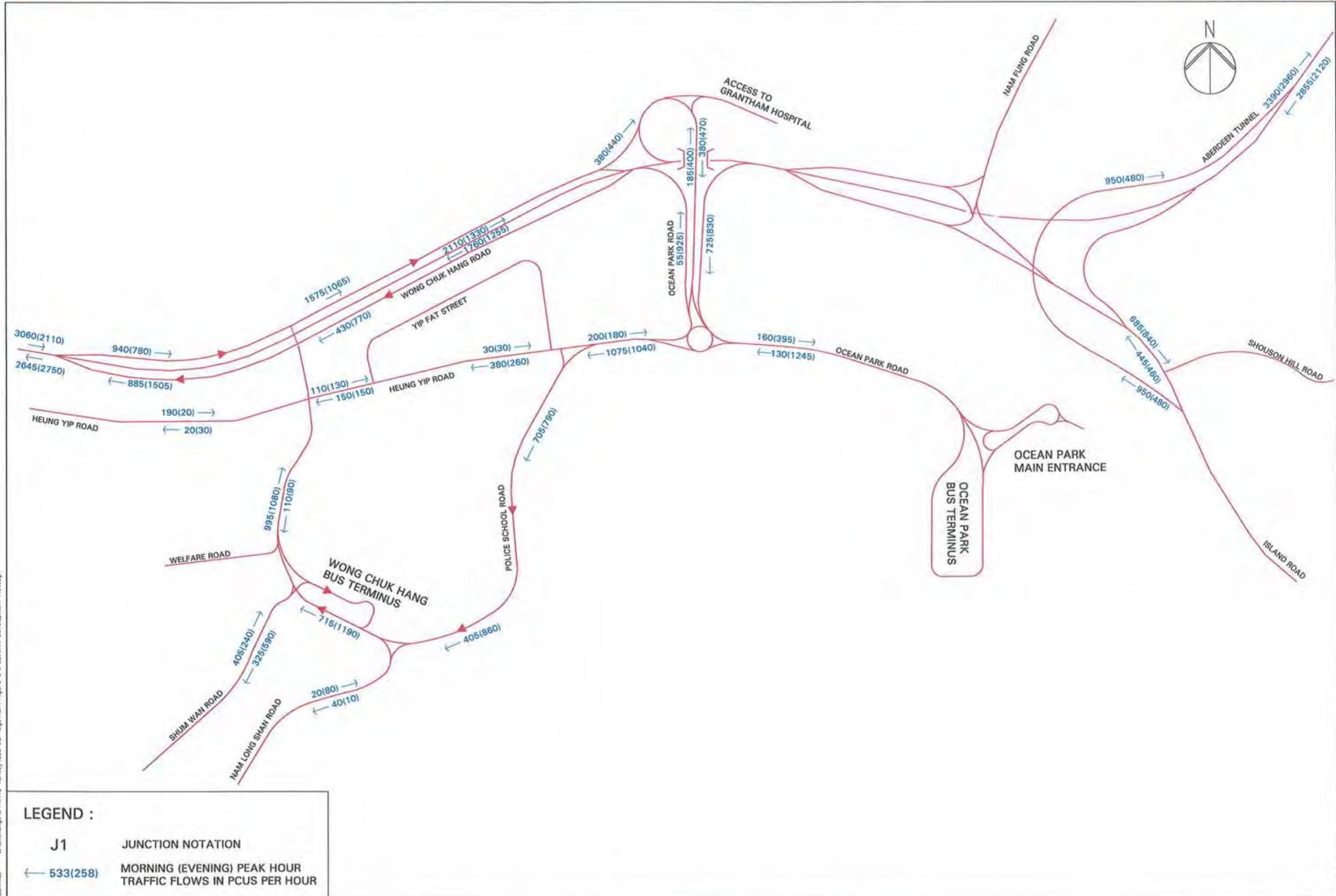
**FIGURE 6.2**

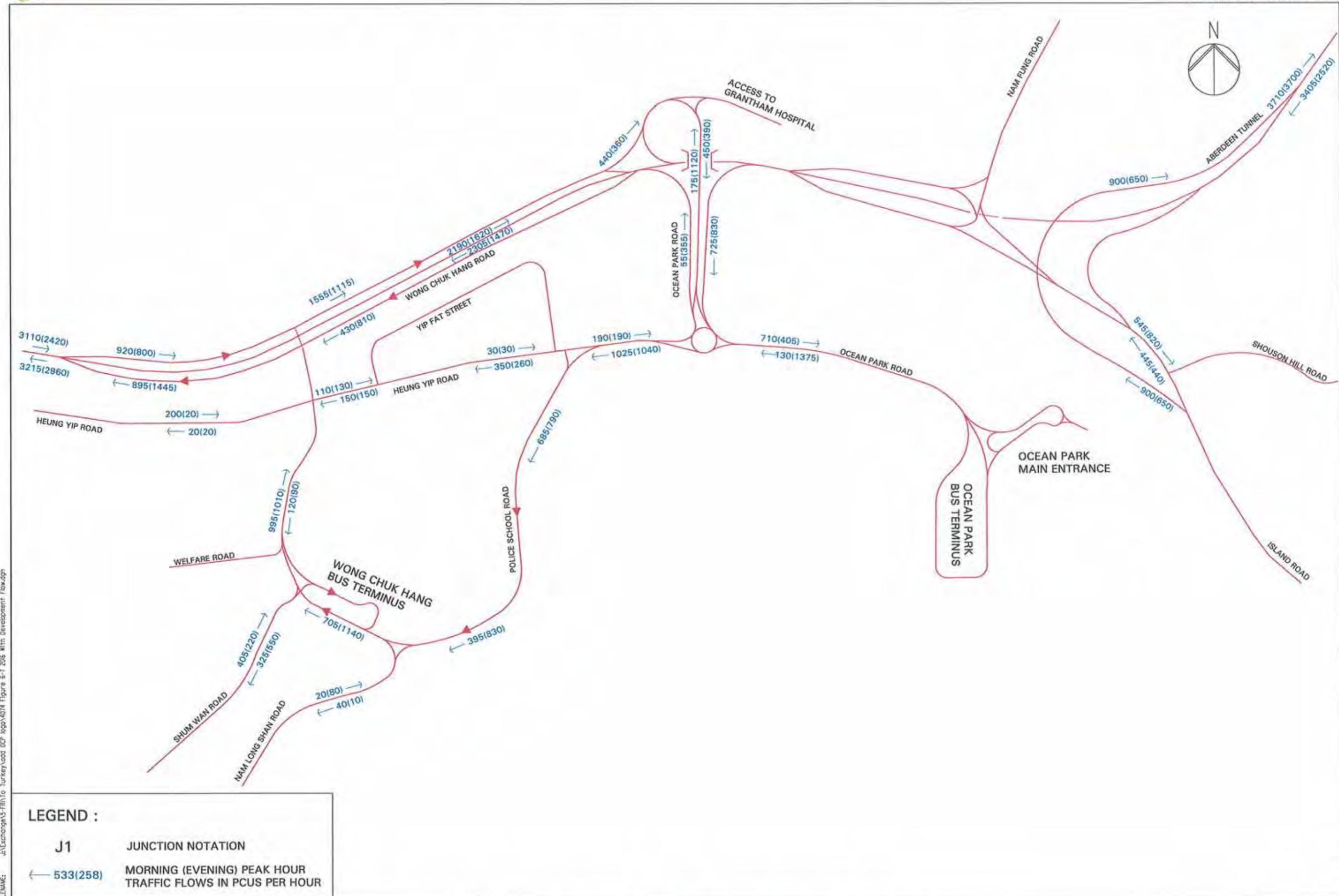


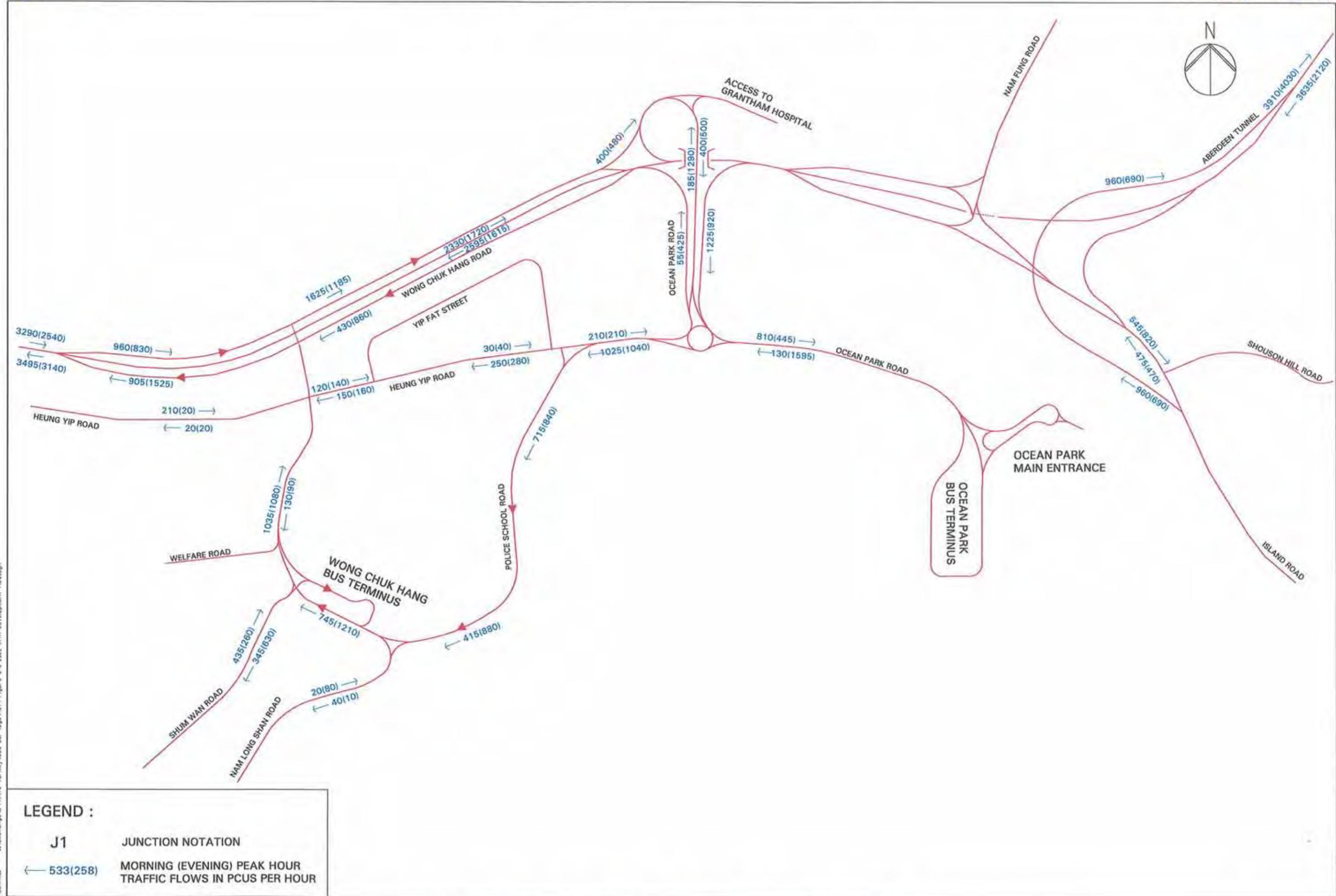




## Feasibility Study for Ocean Park Hotel Development Project







Appendix 6A  
Traffic Signal Calculation Sheets

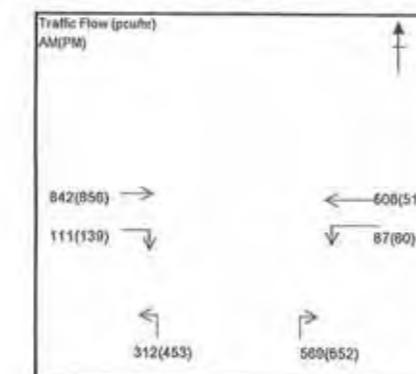
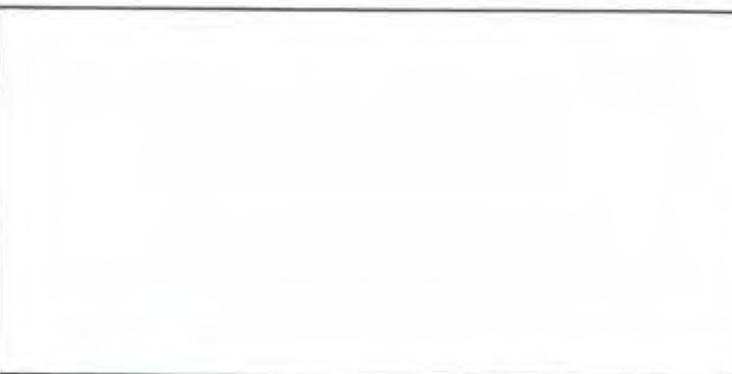
## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

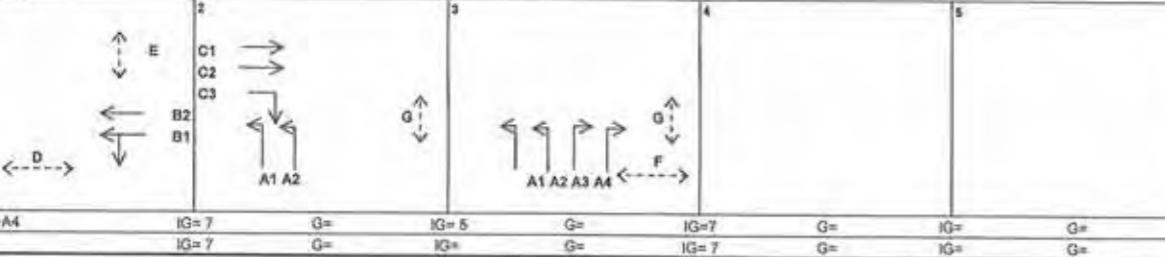
Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

Scheme : Observed Flows

Design Year : 2006  
Designed by: JC  
Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Nam Long Shan Road NB</b>										
A1	2,3	3.30	Y	N	11.0	0.0	148	100%	1712	0.098
A2	2,3	3.30	N	N	15.0	0.0	164	100%	1895	0.086
A3	3	3.30	N	N	29.0	0.0	286	100%	1982	0.144
A4	3	3.30	N	N	25.0	0.0	283	100%	1967	0.144
<b>Wong Chuk Hang Road WB</b>										
B1	1	3.50	Y	N	15.0	0.0	282	31%	1906	0.148
B2	1	3.50	N	N	0.0	0.0	311	0%	2105	0.148
<b>Wong Chuk Hang Road EB</b>										
C1	2	3.30	Y	N	0.0	0.0	408	0%	1945	0.209
C2	2	3.30	N	N	0.0	0.0	436	0%	2085	0.209
C3	2	3.30	N	Y	9.0	0.0	111	100%	1590	0.070
D	1	10GM	+ 12FG	= 22s						
E	1	7GM	+ 8FG	= 15s						
F	3	6GM	+ 9FG	= 15s						
G	2,3	13GM	+ 10FG	= 23s						

Capacity Calculation		AM Peak		PM Peak	
Sum of Critical y Values - Y		B1+A1	B1+C1+A4	B1+A1	B1+C1+A4
Lost Time - L		0.234	0.501	0.269	0.521
Cycle Time - C		12	16	12	16
Practical Capacity - Ypr		100	100	100	100
Reserve Capacity - RC (%)		0.792	0.756	0.792	0.756
		238%	51%	194%	46%

Remark:

Date : 19/Jan/07 Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

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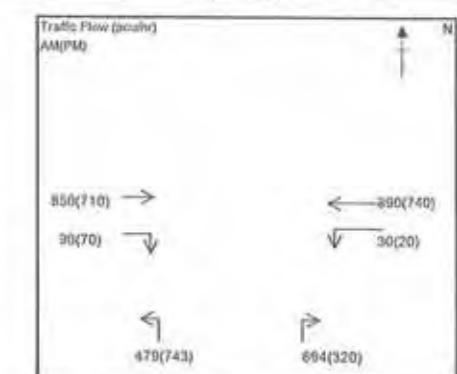
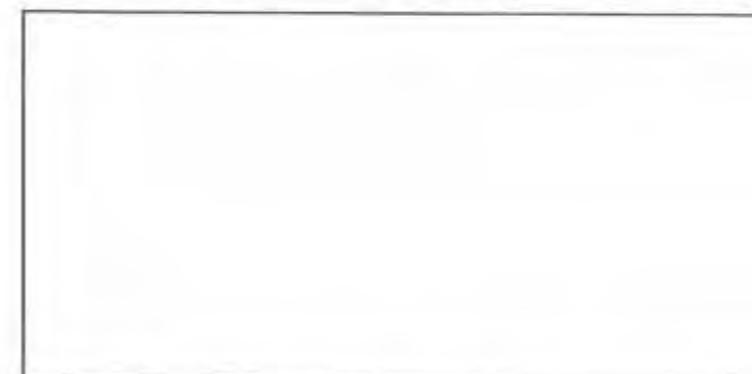
## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

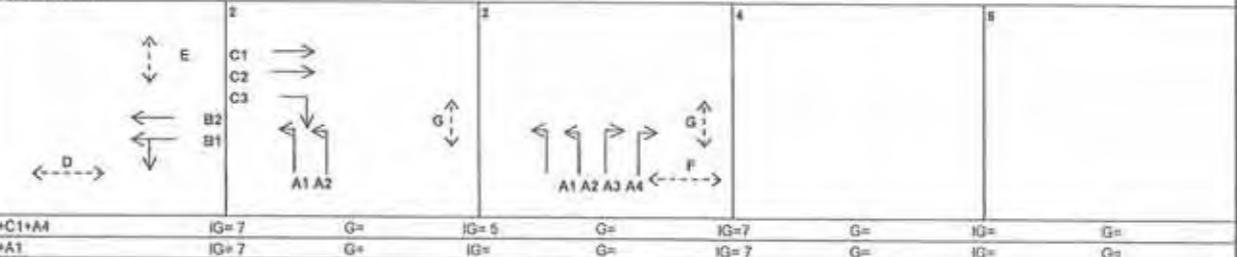
Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

Scheme : Reference (Redevelopment + No Hotel + No SII.)

Design Year : 2011  
Designed by: JC  
Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
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A2	2,3	3.30	N	N	15.0	0.0	252	100%	1895	0.133
A3	3	3.30	N	N	29.0	0.0	348	100%	1982	0.176
A4	3	3.30	N	N	25.0	0.0	346	100%	1987	0.176
<b>Wong Chuk Hang Road WB</b>										
B1	1	3.50	Y	N	15.0	0.0	201	15%	1936	0.104
B2	1	3.50	N	N	0.0	0.0	219	0%	2105	0.104
<b>Wong Chuk Hang Road EB</b>										
C1	2	3.30	Y	N	0.0	0.0	410	0%	1945	0.211
C2	2	3.30	N	N	0.0	0.0	440	0%	2085	0.211
C3	2	3.30	N	Y	9.0	0.0	90	100%	1590	0.057
D	1	10GM	+ 12FG	= 22s						
E	1	7GM	+ 8FG	= 15s						
F	3	6GM	+ 9FG	= 15s						
G	2,3	13GM	+ 10FG	= 23s						

Capacity Calculation		AM Peak		PM Peak	
Sum of Critical y Values - Y		B1+A1	B1+C1+A4	B1+A1	B1+C1+A4
Lost Time - L		0.237	0.491	0.393	0.444
Cycle Time - C		12	16	12	16
Practical Capacity - Ypr		100	100	100	100
Reserve Capacity - RC (%)		0.792	0.756	0.792	0.756
		235%	54%	101%	70%

Remark: Traffic Forecast referenced from Mawson TIA Report (June 2005)

Date : 20/Jun/07 Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

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## APPENDIX 6A

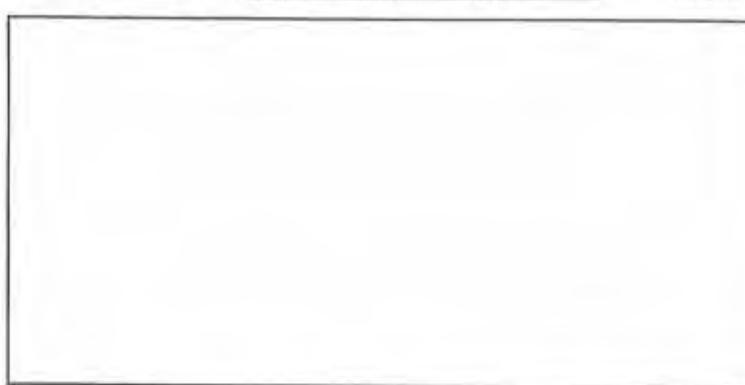
## APPENDIX 6A

## TRAFFIC SIGNAL CALCULATION SHEET

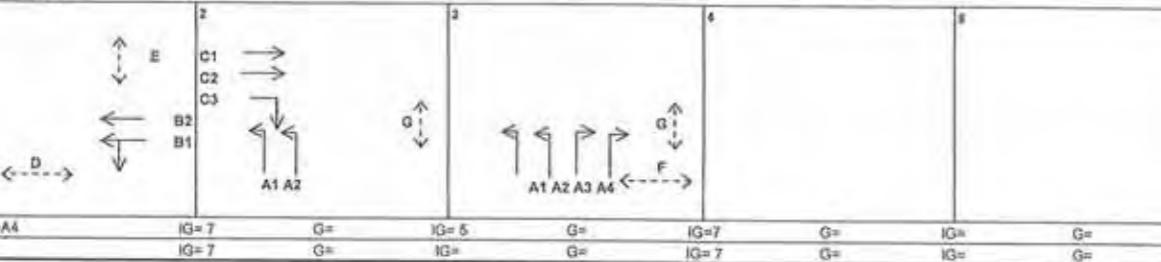
Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

Scheme : Scheme (Redevelopment + New Hotel + No SII.)

Designed by : JC      Checked by : JY



STAGE DIAGRAM



## Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow (pcu/hr) f	Proportion turning (%) r	Saturation flow (pcu/hr) y	PM Peak Flow (pcu/hr) f	Proportion turning (%) r	Saturation flow (pcu/hr) y		
<b>Nam Long Shan Road NB</b>														
A1	2,3	3.30	Y	N	11.0	0.0	231	100%	1712	0.135	357	100%	1712	0.208
A2	2,3	3.30	N	N	15.0	0.0	256	100%	1895	0.135	395	100%	1895	0.208
A3	3	3.30	N	N	29.0	0.0	363	100%	1982	0.183	179	100%	1982	0.090
A4	3	3.30	N	N	25.0	0.0	361	100%	1987	0.183	177	100%	1987	0.090
<b>Wong Chuk Hang Road WB</b>														
B1	1	3.50	Y	N	15.0	0.0	201	15%	1936	0.104	366	5%	1954	0.187
B2	1	3.50	N	N	0.0	0.0	219	0%	2105	0.104	394	0%	2105	0.187
<b>Wong Chuk Hang Road EB</b>														
C1	2	3.30	Y	N	0.0	0.0	410	0%	1945	0.211	343	0%	1945	0.176
C2	2	3.30	N	N	0.0	0.0	440	0%	2085	0.211	367	0%	2085	0.176
C3	2	3.30	N	Y	0.0	0.0	90	100%	1590	0.057	70	100%	1590	0.044
D	1													
E	1													
F	3													
G	2,3													

Capacity Calculation		AM Peak		PM Peak	
Sum of Critical y Values - Y		B1+A1	B1+C1+A4	B1+A1	B1+C1+A4
Lost Time - L		0.239	0.498	0.398	0.454
Cycle Time - C		12	16	12	16
Practical Capacity - Ypr		100	100	100	100
Reserve Capacity - RC (%)		0.792	0.756	0.702	0.756
		231%	52%	100%	67%

Remark: Traffic Forecast referenced from MawSELL TIA Report (June 2005)

Date : 25/Jun/07      Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

ATKINS CHINA LIMITED

## TRAFFIC SIGNAL CALCULATION SHEET

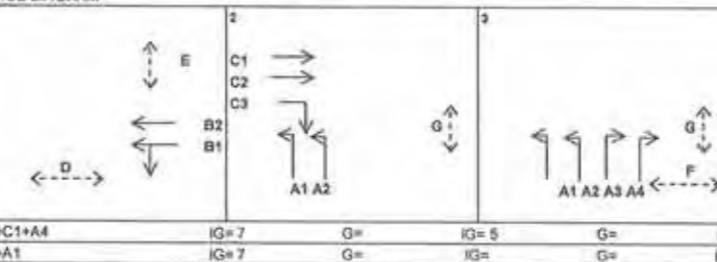
Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

Scheme : Reference (Redevelopment + No Hotel + No SII.)

Designed by : JC      Checked by : JY



STAGE DIAGRAM



## Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow (pcu/hr) f	Proportion turning (%) r	Saturation flow (pcu/hr) y	PM Peak Flow (pcu/hr) f	Proportion turning (%) r	Saturation flow (pcu/hr) y		
<b>Nam Long Shan Road NB</b>														
A1	2,3	3.30	Y	N	11.0	0.0	232	100%	1712	0.136	305	100%	1712	0.178
A2	2,3	3.30	N	N	15.0	0.0	257	100%	1895	0.136	338	100%	1895	0.178
A3	3	3.30	N	N	29.0	0.0	348	100%	1982	0.176	176	100%	1982	0.088
A4	3	3.30	N	N	25.0	0.0	346	100%	1987	0.176	174	100%	1987	0.088
<b>Wong Chuk Hang Road WB</b>														
B1	1	3.50	Y	N	15.0	0.0	201	15%	1936	0.104	365	5%	1955	0.197
B2	1	3.50	N	N	0.0	0.0	219	0%	2105	0.104	415	0%	2105	0.197
<b>Wong Chuk Hang Road EB</b>														
C1	2	3.30	Y	N	0.0	0.0	401	0%	1945	0.206	352	0%	1945	0.181
C2	2	3.30	N	N	0.0	0.0	429	0%	2085	0.206	378	0%	2085	0.181
C3	2	3.30	N	Y	0.0	0.0	90	100%	1590	0.057	70	100%	1590	0.044
D	1													
E	1													
F	3													
G	2,3													

Capacity Calculation		AM Peak		PM Peak	
Sum of Critical y Values - Y		B1+A1	B1+C1+A4	B1+A1	B1+C1+A4
Lost Time - L		0.239	0.486	0.375	0.467
Cycle Time - C		12	16	12	16
Practical Capacity - Ypr		100	100	100	100
Reserve Capacity - RC (%)		0.792	0.756	0.792	0.756
		231%	52%	66%	62%

Remark: Traffic Forecast referenced from MawSELL TIA Report (June 2005)

Date : 10/Jun/07      Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

ATKINS CHINA LIMITED

## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

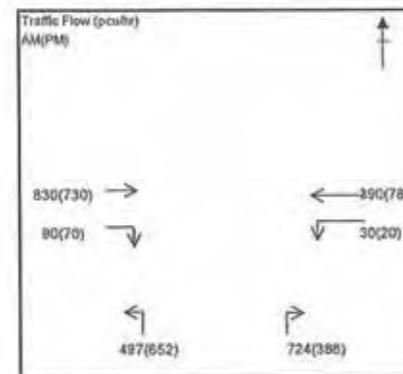
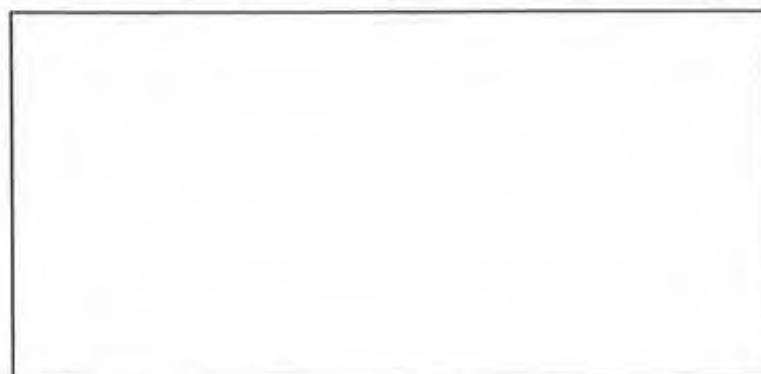
Scheme : Scheme (Redevelopment + New Hotel + No SIL)

JOB NO. : 4074

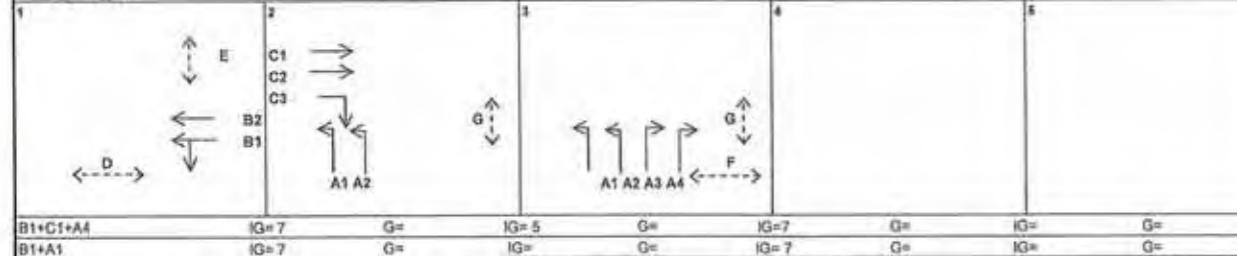
Design Year: 2016

Designed by: JC

Checked by: JV



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Nam Long Shan Road NB</b>										
A1	2,3	3.30	Y	N	11.0	0.0	236	100%	1712	0.138
A2	2,3	3.30	N	N	15.0	0.0	261	100%	1895	0.138
A3	3	3.30	N	N	29.0	0.0	383	100%	1982	0.183
A4	3	3.30	N	N	25.0	0.0	361	100%	1967	0.183
<b>Wong Chuk Hang Road WB</b>										
B1	1	3.50	Y	N	15.0	0.0	201	15%	1938	0.104
B2	1	3.50	N	N	0.0	0.0	219	0%	2105	0.104
<b>Wong Chuk Hang Road EB</b>										
C1	2	3.30	Y	N	0.0	0.0	401	0%	1945	0.206
C2	2	3.30	N	N	0.0	0.0	429	0%	2085	0.206
C3	2	3.30	N	Y	9.0	0.0	90	100%	1590	0.057
D	1									
E	1									
F	3									
G	2,3									
		10GM	+ 12FG	= 22s						
		7GM	+ 8FG	= 15s						
		6GM	+ 9FG	= 15s						
		13GM	+ 10FG	= 23s						

Capacity Calculation	AM Peak		PM Peak	
	B1+A1	B1+C1+A4	B1+A1	B1+C1+A4
Sum of Critical y Values - Y	0.242	0.493	0.378	0.476
Lost Time - L	12	16	12	16
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.792	0.756	0.792	0.756
Reserve Capacity - RC (%)	228%	63%	110%	69%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 19/Jun/07 Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

ATKINS CHINA LIMITED

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

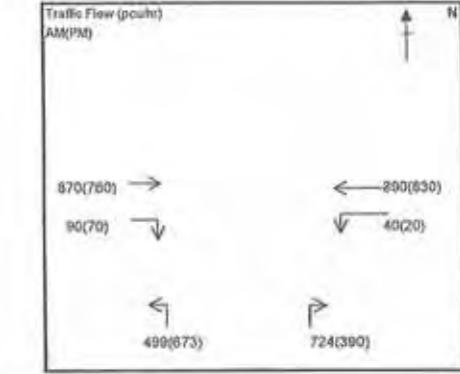
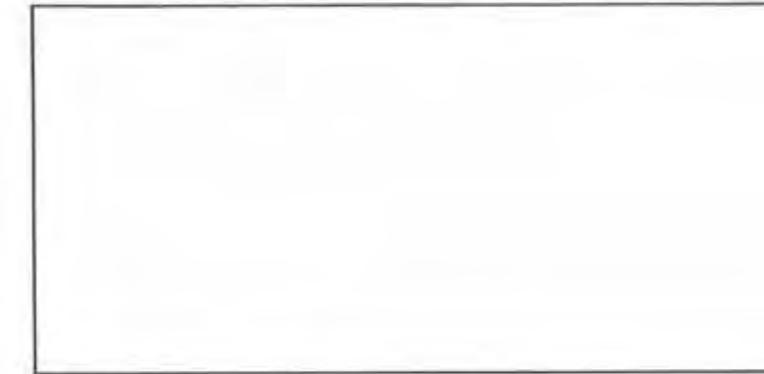
Scheme : Reference (Redevelopment + No Hotel + No SIL)

JOB NO. : 4074

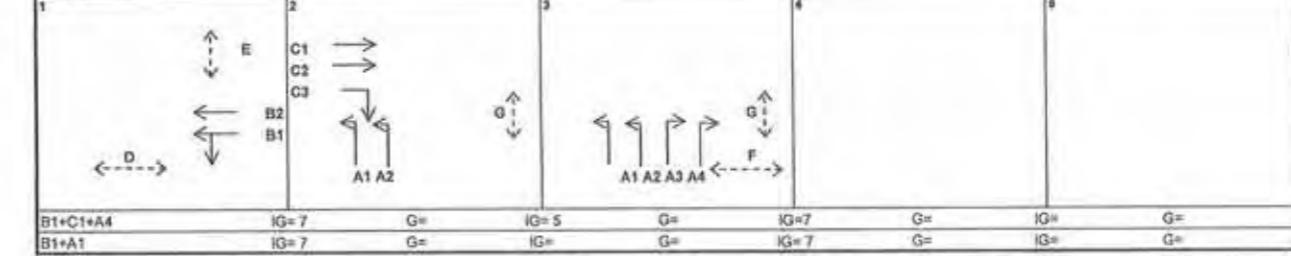
Design Year: 2022

Designed by: JC

Checked by: JV



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Nam Long Shan Road NB</b>										
A1	2,3	3.30	Y	N	11.0	0.0	237	100%	1712	0.138
A2	2,3	3.30	N	N	15.0	0.0	262	100%	1895	0.138
A3	3	3.30	N	N	29.0	0.0	383	100%	1982	0.183
A4	3	3.30	N	N	25.0	0.0	361	100%	1967	0.183
<b>Wong Chuk Hang Road WB</b>										
B1	1	3.50	Y	N	15.0	0.0	206	19%	1927	0.107
B2	1	3.50	N	N	0.0	0.0	224	0%	2105	0.107
<b>Wong Chuk Hang Road EB</b>										
C1	2	3.30	Y	N	0.0	0.0	420	0%	1945	0.216
C2	2	3.30	N	N	0.0	0.0	450	0%	2085	0.216
C3	2	3.30	N	Y	9.0	0.0	90	100%	1590	0.057
D	1									
E	1									
F	3									
G	2,3									
		10GM	+ 12FG	= 22s						
		7GM	+ 8FG	= 15s						
		6GM	+ 9FG	= 15s						
		13GM	+ 10FG	= 23s						

Capacity Calculation	AM Peak		PM Peak	
	B1+A1	B1+C1+A4	B1+A1	B1+C1+A4
Sum of Critical y Values - Y	0.245	0.506	0.386	0.49

## APPENDIX 6A

## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

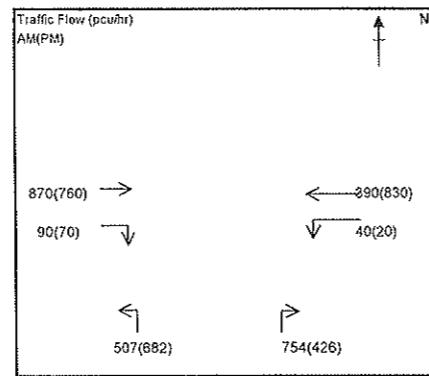
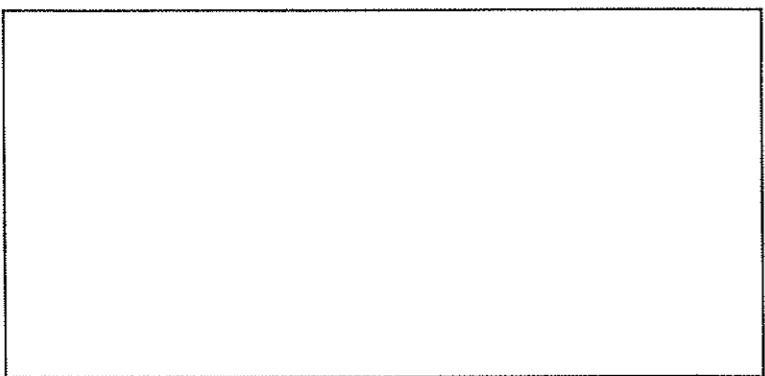
JOB NO. : 4074

Design Year: 2022

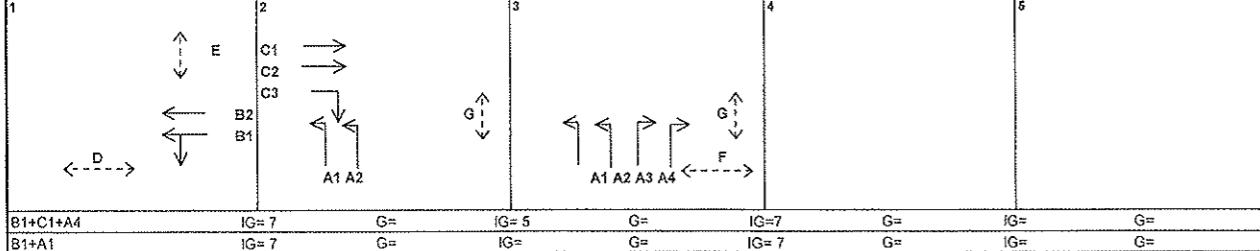
Scheme : Scheme (Redevelopment + New Hotel + No SIL)

Designed by: JC

Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations					
							AM Peak Flow (pcu/hr)	Proportion f	Saturation y value	PM Peak Flow (pcu/hr)	Proportion f	Saturation y value		
<b>Nam Long Shan Road NB</b>														
A1	2,3	3.30	Y	N	11.0	0.0	241	100%	1712	0.141	324	100%	1712	0.189
A2	2,3	3.30	N	N	15.0	0.0	206	100%	1895	0.141	358	100%	1895	0.189
A3	3	3.30	N	N	29.0	0.0	378	100%	1982	0.191	214	100%	1982	0.108
A4	3	3.30	N	N	25.0	0.0	376	100%	1967	0.191	212	100%	1967	0.108
<b>Wong Chuk Hang Road WB</b>														
B1	1	3.50	Y	N	15.0	0.0	206	19%	1927	0.107	409	5%	1955	0.209
B2	1	3.50	N	N	0.0	0.0	224	0%	2105	0.107	441	0%	2105	0.209
<b>Wong Chuk Hang Road EB</b>														
C1	2	3.30	Y	N	0.0	0.0	420	0%	1945	0.216	367	0%	1945	0.189
C2	2	3.30	N	N	0.0	0.0	450	0%	2085	0.216	393	0%	2085	0.189
C3	2	3.30	N	Y	9.0	0.0	90	100%	1590	0.057	70	100%	1590	0.044
D	1				10GM	+ 12FG	= 22s							
E	1				7GM	+ 8FG	= 15s							
F	3				6GM	+ 9FG	= 15s							
G	2,3				13GM	+ 10FG	= 23s							

Capacity Calculation		AM Peak		PM Peak	
Sum of Critical y Values - Y		B1+A1	0.247	B1+C1+A4	0.613
Lost Time - L		12	16	12	16
Cycle Time - C		100	100	100	100
Practical Capacity - Ypr		0.792	0.756	0.792	0.756
Reserve Capacity - RC (%)		220%	47%	99%	49%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 19/Jan/07

Junction : J1 Wong Chuk Hang Road/ Nam Long Shan Road (W14)

ATKINS CHINA LIMITED

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

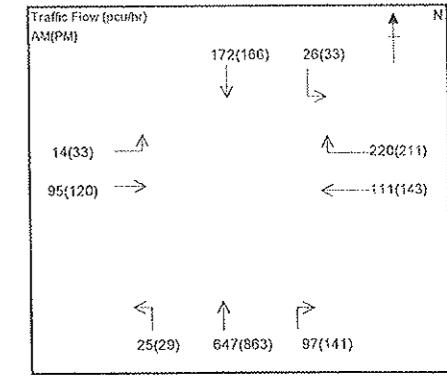
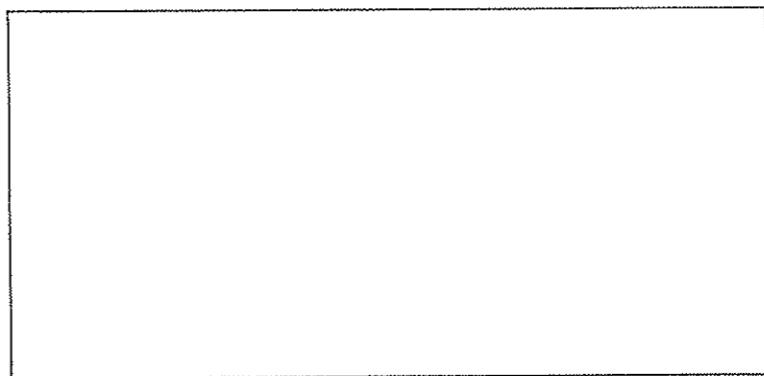
JOB NO. : 4074

Design Year: 2006

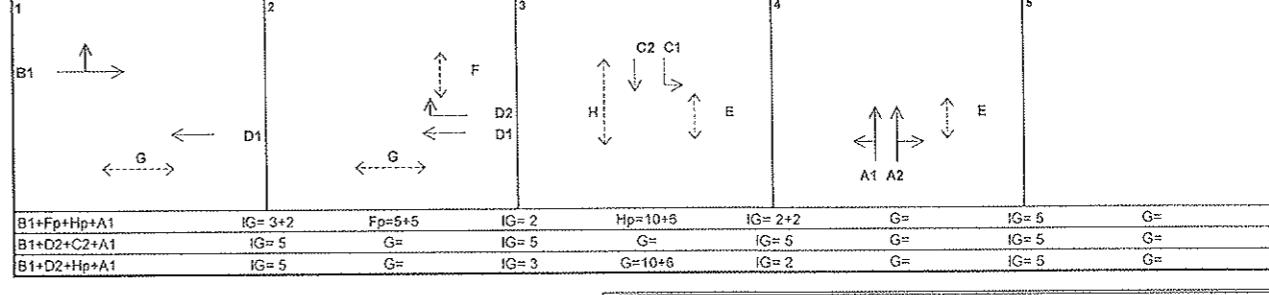
Scheme : Observed Flows

Designed by: JC

Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations					
							AM Peak Flow (pcu/hr)	Proportion f	Saturation y value	PM Peak Flow (pcu/hr)	Proportion f	Saturation y value		
<b>Nam Long Shan Road NB</b>														
A1	4	3.50	Y	N	6.0	0.0	374	7%	1933	0.193	503	6%	1937	0.260
A2	4	3.50	N	N	12.0	0.0	395	25%	2042	0.193	530	27%	2037	0.260
<b>Heung Yip Road EB</b>														
B1	1	4.00	Y	N	9.0	0.0	109	13%	1973	0.055	153	22%	1945	0.079
<b>Nam Long Shan Road SB</b>														
C1	3	3.50	Y	N	6.0	0.0	26	100%	1572	0.017	33	100%	1572	0.021
C2	3	3.50	N	N	0.0	0.0	172	0%	2105	0.082	166	0%	2105	0.079
<b>Heung Yip Road WB</b>														
D1	1,2	3.25	Y	N	0.0	0.0	111	0%	1940	0.057	143	0%	1940	0.074
D2	2	3.25	N	N	9.0	0.0	220	100%	1783	0.123	211	100%	1783	0.118
<b>Ep</b>														
Fp	2	5GM	+ 6FG	= 11s										
Gp	1,2	5GM	+ 5FG	= 10s										
Hp	3	10GM	+ 9FG	= 19s										
		10GM	+ 6FG	= 16s										

Capacity Calculation		AM Peak		PM Peak	
Sum of Critical y Values - Y					

APPENDIX 6A

APPENDIX 6A

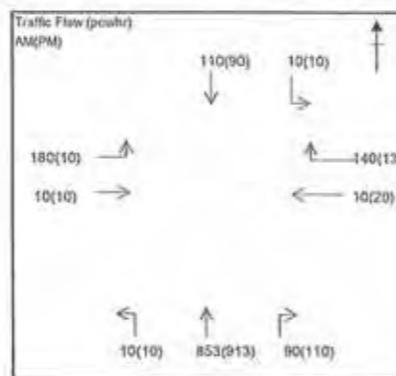
## TRAFFIC SIGNAL CALCULATION SHEET

Junction : J2 - Heung Yip Road / Nam Long Shan Road (W15)

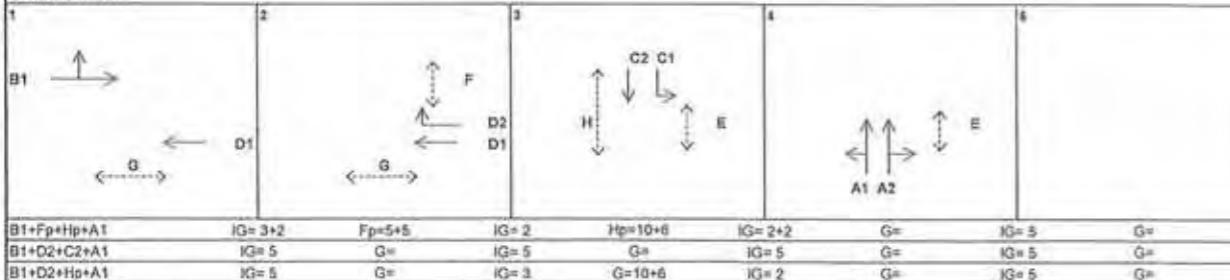
Schema : \_\_\_\_\_ Reference (Redevelopment + No Hotel + No

Design Year: 2011

Design Year: 2011



**STAGE DIAGRAM**



## Saturation Flow Calculations

Capacity Calculation	AM Peak		PM Peak	
	B1+D2+H+A1	B1+D2+C2+A1	B1+D2+H+A1	B1+D2+C2+A1
Sum of Critical y Values - Y	0.425	0.477	0.341	0.384
Lost Time - L	28	16	28	15
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.645	0.756	0.648	0.756
Reserve Capacity - RC (%)	52%	58%	50%	57%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07 Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

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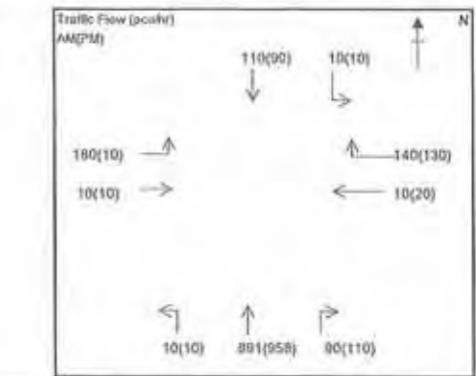
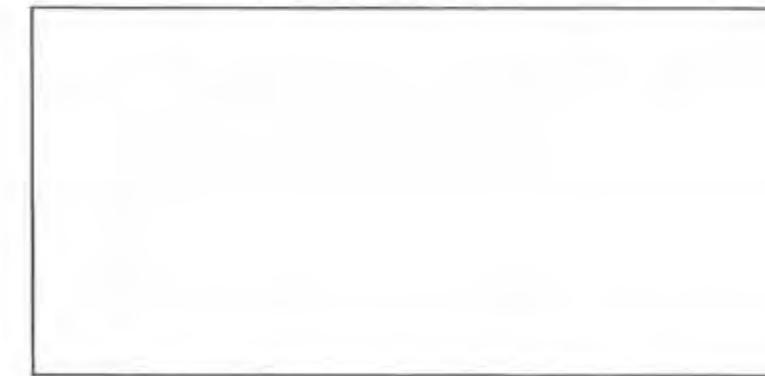
TRAFFIC SIGNAL CALCULATION SHEET

Junction J2 - Heung Yip Road/ Nam Long Shan Road (W15)

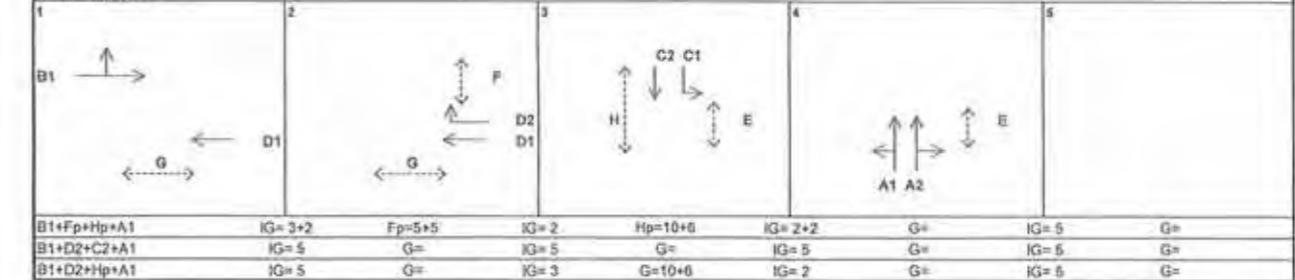
Scheme \_\_\_\_\_ Scheme (Redevelopment + New Hotel + No SII)

JOB NO.: 4074

Design Year: 2011



**STAGE DIAGRAM**



## Saturation Flow Calculations

Capacity Calculation	AM Peak		PM Peak	
	B1+D2+Hp+A1	B1+D2+C2+A1	B1+D2+H+A1	B1+D2+C2+A1
Sum of Critical Values - Y	0.435	0.487	0.353	0.395
Lost Time - L	28	16	28	18
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.648	0.758	0.648	0.758
Reserve Capacity - RC (%)	49%	55%	84%	91%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07 Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

ATKINS CHINA LIMITED

APPENDIX 6A

APPENDIX 6A

## TRAFFIC SIGNAL CALCULATION SHEET

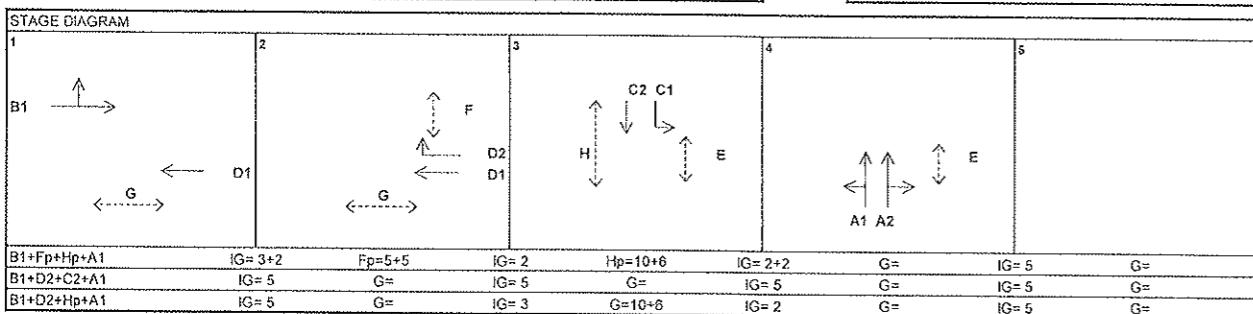
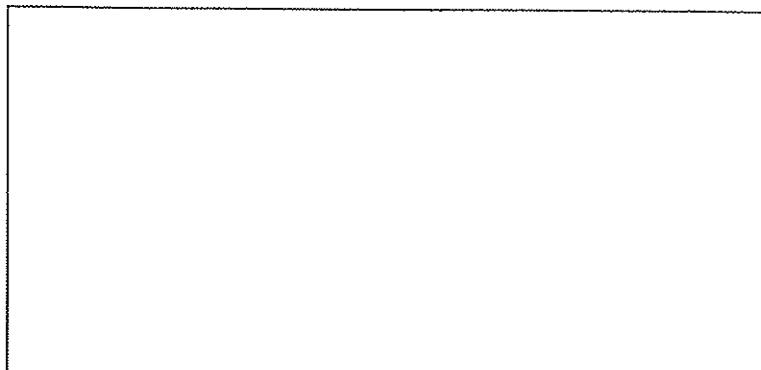
Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

Scheme : Reference /Redevelopment + No Hotel + No S

Designated by

JOB NO. : 4074

Design Year: 2016



## Saturation Flow Calculations

Capacity Calculation	AM Peak		PM Peak	
	B1+D2+Hp+A1	B1+D2+C2+A1	B1+D2+H+A1	B1+D2+C2+A1
Sum of Critical Y Values - Y	0.417	0.474	0.313	0.356
Lost Time - L	28	16	28	16
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.648	0.756	0.648	0.756
Reserve Capacity - RC (%)	55%	59%	107%	112%

**Remark:** Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/2018

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

ATKINS CHINA LIMITED

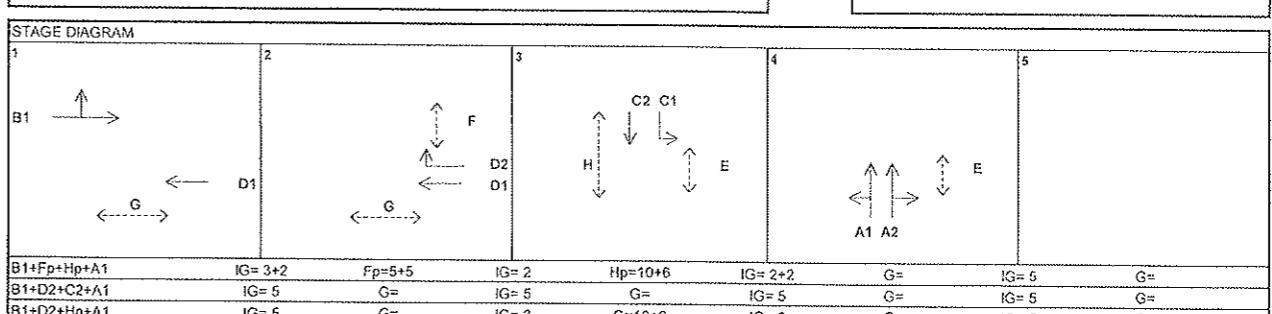
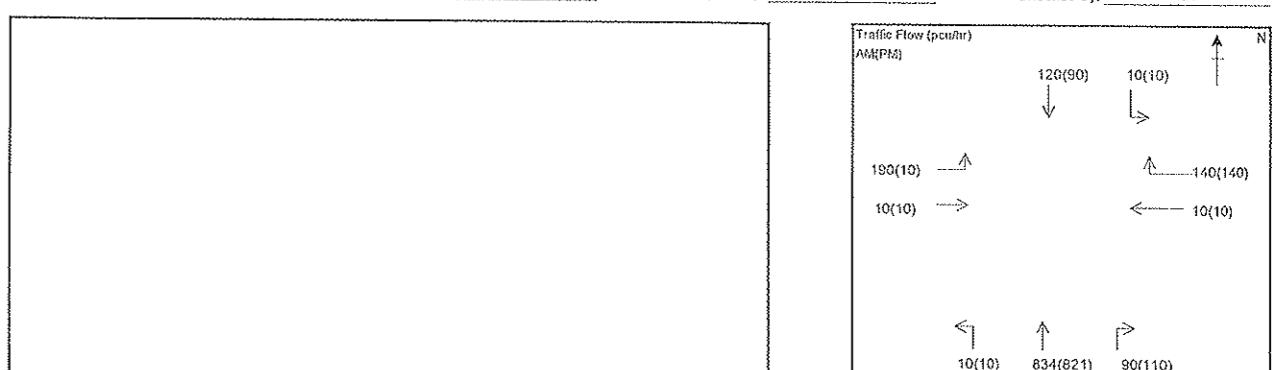
## TRAFFIC SIGNAL CALCULATION SHEET

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

JOB NO.: 402

4074

2016



## Saturation Flow Calculations

Capacity Calculation	AM Peak		PM Peak	
	B1+D2+Hp+A1	B1+D2+C2+A1	B1+D2+H+A1	B1+D2+C2+A1
Sum of Critical y Values - Y	0.426	0.483	0.324	0.367
Lost Time - L	28	16	28	16
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.648	0.756	0.648	0.756
Reserve Capacity - RC (%)	52%	56%	100%	105%

Remark: Traffic Forecast referenced from Mavinsell TIA Report (June 2005)

Date : 25/Jan/07

Junction : J2 - Heung Yin Road/ Nam Long Shan Road (W15)

ATKINS CHINA LIMITED

## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

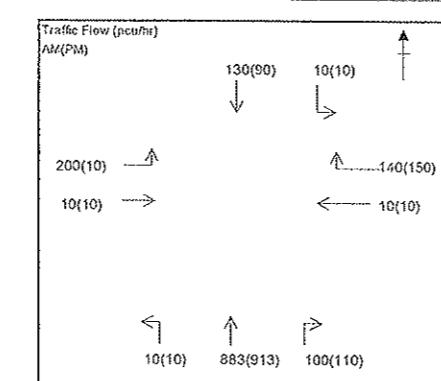
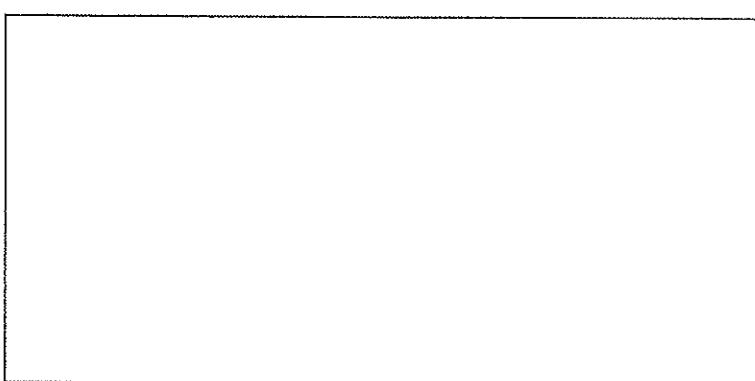
Scheme : Reference (Redevelopment + No Hotel + No SIL)

JOB NO. : 4074

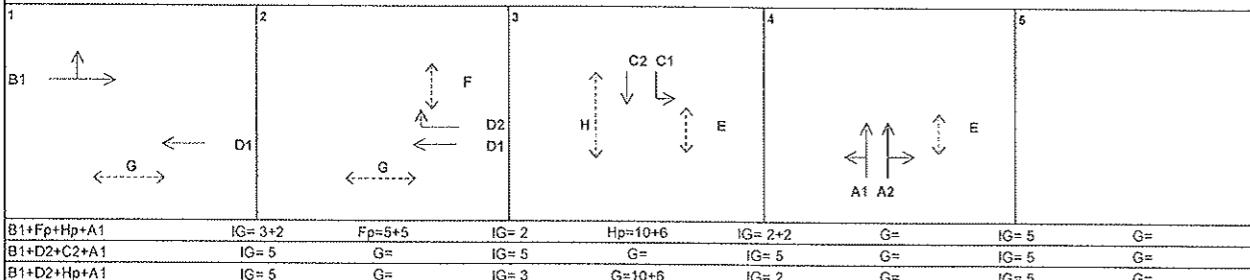
Design Year: 2022

Designed by: JC

Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Nam Long Shan Road NB</b>										
A1	4	3.50	Y	N	6.0	0.0	484	2%	1955	0.248
A2	4	3.50	N	N	12.0	0.0	509	20%	2056	0.248
<b>Heung Yip Road EB</b>										
B1	1	4.00	Y	N	9.0	0.0	210	95%	1739	0.121
<b>Nam Long Shan Road SB</b>										
C1	3	3.50	Y	N	6.0	0.0	10	100%	1572	0.006
C2	3	3.50	N	N	0.0	0.0	130	0%	2105	0.062
<b>Heung Yip Road WB</b>										
D1	1,2	3.25	Y	N	0.0	0.0	10	0%	1940	0.005
D2	2	3.25	N	N	9.0	0.0	140	100%	1783	0.079
<b>Ep</b>										
Ep	3,4	5GM	+ 6FG	= 11s						
Fp	2	5GM	+ 5FG	= 10s						
Gp	1,2	10GM	+ 9FG	= 19s						
Hp	3	10GM	+ 6FG	= 16s						

Capacity Calculation	AM Peak		PM Peak	
	B1+D2+Hp+A1	B1+D2+C2+A1	B1+D2+H+A1	B1+D2+C2+A1
Sum of Critical y Values - Y	0.447	0.503	0.353	0.395
Lost Time - L	28	16	28	16
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.648	0.756	0.648	0.756
Reserve Capacity - RC (%)	45%	49%	84%	91%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

ATKINS CHINA LIMITED

## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

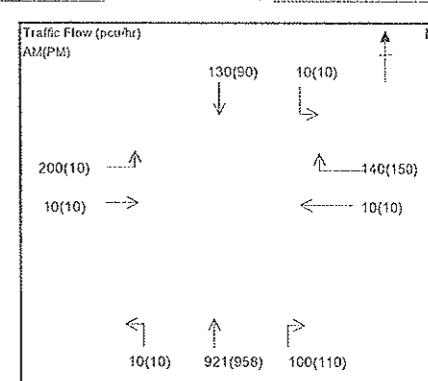
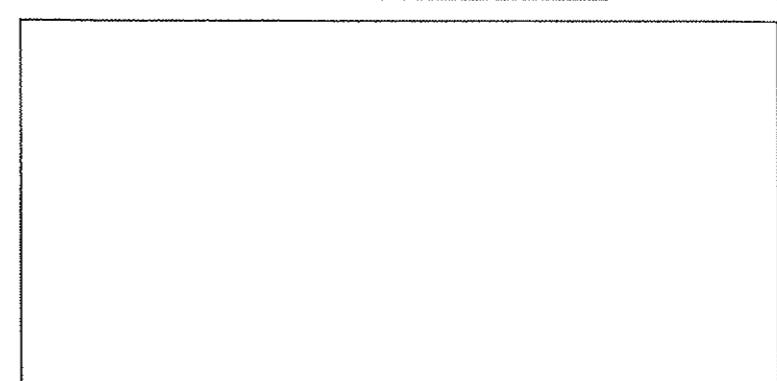
Scheme : Scheme (Redevelopment + New Hotel + No SIL)

JOB NO. : 4074

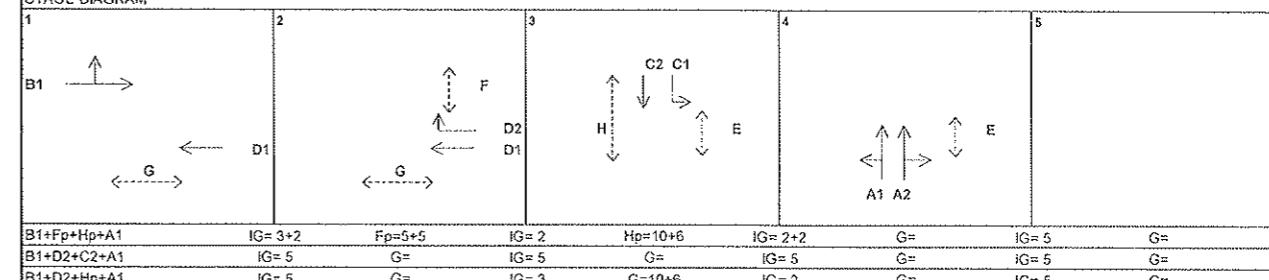
Design Year: 2022

Designed by: JC

Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Nam Long Shan Road NB</b>										
A1	4	3.50	Y	N	6.0	0.0	503	2%	1955	0.257
A2	4	3.50	N	N	12.0	0.0	528	19%	2056	0.257
<b>Heung Yip Road EB</b>										
B1	1	4.00	Y	N	9.0	0.0	210	95%	1739	0.121
<b>Nam Long Shan Road SB</b>										
C1	3	3.50	Y	N	6.0	0.0	10	100%	1572	0.006
C2	3	3.50	N	N	0.0	0.0	130	0%	2105	0.062
<b>Heung Yip Road WB</b>										
D1	1,2	3.25	Y	N	0.0	0.0	10	0%	1940	0.005
D2	2	3.25	N	N	9.0	0.0	140	100%	1783	0.079
<b>Ep</b>										
Ep	3,4	5GM	+ 6FG	= 11s						
Fp	2	5GM	+ 5FG	= 10s						
Gp	1,2	10GM	+ 9FG	= 19s						
Hp	3	10GM	+ 6FG	= 16s						

Capacity Calculation	AM Peak		PM Peak	
	B1+D2+Hp+A1	B1+D2+C2+A1	B1+D2+H+A1	B1+D2+C2+A1
Sum of Critical y Values - Y	0.456	0.518	0.364	0.407
Lost Time - L	28	16	28	16
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.648	0.756	0.648	0.756
Reserve Capacity - RC (%)	42%	46%	78%	86%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07

Junction : J2 - Heung Yip Road/ Nam Long Shan Road (W15)

ATKINS CHINA LIMITED

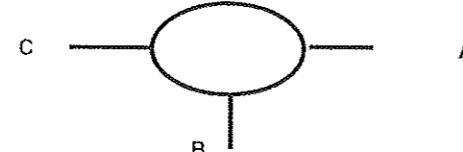
## APPENDIX 6A

### Simplified Roundabout Capacity Calculation

**ATKINS**

Job Title:	Ocean Park Hotel Development Feasibility Study
Junction:	J3 - Ocean Park Road / Heung Yip Road Roundabout
Scheme:	Observed Flows
Design Year:	2006
Job No.:	4074
Designed by:	JC
Checked by:	JY
Date :	1/25/2007

ARM A: Heung Yip Road  
 ARM B: Ocean Park Road (to WCH Rd)  
 ARM C: Ocean Park Road (to Main Entrance)



GEOMETRY							
ARM	V	e	L	r	D	Phi	S
A	5.50	8.00	25	12	37	15	0.16
B	11.00	12.50	20	33	37	17.5	0.12
C	5.50	11.00	27.5	100	37	15	0.32

AM FLOWS							
from/to	A	B	C	Circ	Entry		
A	11	216	29	304	256		
B	1024	302	217	42	1543		
C	63	0	2	1337	65		

Flow in pcu/hr

PM FLOWS							
from/to	A	B	C	Circ	Entry		
A	4	212	85	386	301		
B	990	318	137	98	1445		
C	141	59	9	1312	208		

Flow in pcu/hr

CALCULATIONS									
ARM	K	X <sub>2</sub>	M	F	t <sub>b</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM)	Q <sub>E</sub> (PM)	RFC
A	1.02	7.39	0.10	2240	1.45	0.76	2049	1986	0.12
B	1.06	12.21	0.10	3700	1.45	1.05	3884	3822	0.40
C	1.09	8.85	0.10	2683	1.45	0.85	1693	1716	0.04

Critical Arm: B      RFC: 0.40      0.38

- In accordance with TPDM V2.4

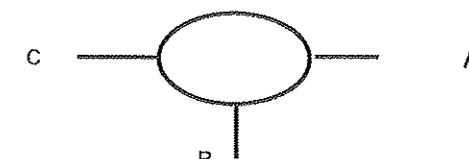
### Simplified Roundabout Capacity Calculation

APPENDIX 6A

**ATKINS**

Job Title:	Ocean Park Hotel Development Feasibility Study
Junction:	J3 - Ocean Park Road / Heung Yip Road Roundabout
Scheme:	Reference (Redevelopment+ No Hotel+ No SIL)
Design Year:	2011
Job No.:	4074
Designed by:	JC
Checked by:	JY
Date :	1/25/2007

ARM A: Heung Yip Road  
 ARM B: Ocean Park Road (to WCH Rd)  
 ARM C: Ocean Park Road (to Main Entrance)



GEOMETRY							
ARM	V	e	L	r	D	Phi	S
A	5.50	8.00	25	12	37	15	0.16
B	11.00	12.50	20	33	37	17.5	0.12
C	5.50	11.00	27.5	100	37	15	0.32

AM FLOWS							
from/to	A	B	C	Circ	Entry		
A	10	90	100	118	200		
B	1013	40	8	120	1061		
C	10	68	10	1063	88		

PM FLOWS							
from/to	A	B	C	Circ	Entry		
A	10	50	120	1241	180		
B	953	70	221	140	1244		
C	30	1161	10	1033	1201		

CALCULATIONS									
ARM	K	X <sub>2</sub>	M	F	t <sub>b</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM)	Q <sub>E</sub> (PM)	RFC
A	1.02	7.39	0.10	2240	1.45	0.76	2193	1326	0.09
B	1.06	12.21	0.10	3700	1.45	1.05	3797	3775	0.28
C	1.09	8.85	0.10	2683	1.45	0.85	1946	1973	0.05

Critical Arm: B      RFC: 0.28      0.61

- In accordance with TPDM V2.4

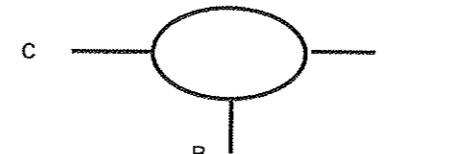
## APPENDIX 6A

### Simplified Roundabout Capacity Calculation

**ATKINS**

Job Title:	Ocean Park Hotel Development Feasibility Study	
Junction:	J3 - Ocean Park Road / Heung Yip Road Roundabout	Designed by: JC
Scheme:	Scheme (Redevelopment+ New Hotel+ No SIL)	Checked by: JY
Design Year:	2011	Job No.: 4074
	Date :	1/25/2007

ARM A: Heung Yip Road  
 ARM B: Ocean Park Road (to WCH Rd)  
 ARM C: Ocean Park Road (to Main Entrance)



GEOMETRY							
ARM	V	e	L	r	D	Phi	S
A	5.50	8.00	25	12	37	15	0.16
B	11.00	12.50	20	33	37	17.5	0.12
C	5.50	11.00	27.5	100	37	15	0.32

AM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	90	100	157	200
B	1051	40	47	120	1138
C	10	107	10	1101	127

Flow in pcu/hr

PM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	50	120	1287	180
B	998	70	267	140	1335
C	30	1207	10	1078	1247

Flow in pcu/hr

CALCULATIONS										RFC	
ARM	K	X <sub>2</sub>	M	F	t <sub>d</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM)	Q <sub>E</sub> (PM)	AM	PM	
A	1.02	7.39	0.10	2240	1.45	0.76	2163	1291	0.09	0.14	
B	1.06	12.21	0.10	3700	1.45	1.05	3797	3775	0.30	0.35	
C	1.09	8.85	0.10	2683	1.45	0.85	1911	1932	0.07	0.65	

Critical Arm:      B      C  
 RFC:      0.30      0.65

- In accordance with TPDM V2.4

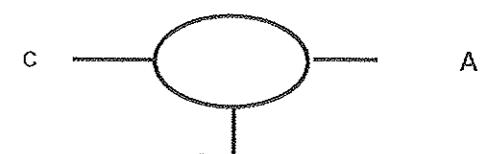
## APPENDIX 6A

### Simplified Roundabout Capacity Calculation

**ATKINS**

Job Title:	Ocean Park Hotel Development Feasibility Study	
Junction:	J3 - Ocean Park Road / Heung Yip Road Roundabout	Designed by: JC
Scheme:	Reference (Redevelopment+ No Hotel+ No SIL)	Checked by: JY
Design Year:	2016	Job No.: 4074
	Date :	1/25/2007

ARM A: Heung Yip Road  
 ARM B: Ocean Park Road (to WCH Rd)  
 ARM C: Ocean Park Road (to Main Entrance)



GEOMETRY							
ARM	V	e	L	r	D	Phi	S
A	5.50	8.00	25	12	37	15	0.16
B	11.00	12.50	20	33	37	17.5	0.12
C	5.50	11.00	27.5	100	37	15	0.32

AM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	80	100	118	190
B	963	40	558	120	1561
C	10	68	10	1013	88

PM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	60	120	1381	190
B	973	70	231	140	1274
C	20	1301	10	1053	1331

CALCULATIONS										RFC	
ARM	K	X <sub>2</sub>	M	F	t <sub>d</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM)	Q <sub>E</sub> (PM)	AM	PM	
A	1.02	7.39	0.10	2240	1.45	0.76	2193	1218	0.09	0.16	
B	1.06	12.21	0.10	3700	1.45	1.05	3797	3775	0.41	0.34	
C	1.09	8.85	0.10	2683	1.45	0.85	1992	1955	0.04	0.68	

Critical Arm:      B      C  
 RFC:      0.41      0.68

- In accordance with TPDM V2.4

## APPENDIX 6A

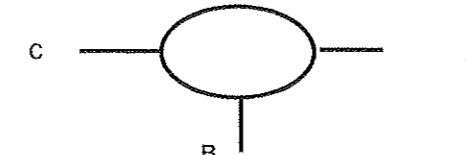
### Simplified Roundabout Capacity Calculation

**ATKINS**

Job Title:	Ocean Park Hotel Development Feasibility Study
Junction:	J3 - Ocean Park Road / Heung Yip Road Roundabout
Scheme:	Scheme (Redevelopment+ New Hotel+ No SIL)
Design Year:	2016

Job No.:	4074	Date :	1/25/2007
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ARM A: Heung Yip Road  
 ARM B: Ocean Park Road (to WCH Rd)  
 ARM C: Ocean Park Road (to Main Entrance)



GEOMETRY							
ARM	v	e	L	r	D	Phi	S
A	5.50	8.00	25	12	37	15	0.16
B	11.00	12.50	20	33	37	17.5	0.12
C	5.50	11.00	27.5	100	37	15	0.32

AM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	80	100	157	190
B	1001	40	597	120	1638
C	10	107	10	1051	127

Flow in pcu/hr

PM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	60	120	1427	190
B	1018	70	277	140	1365
C	20	1347	10	1098	1377

Flow in pcu/hr

CALCULATIONS									
ARM	K	X <sub>2</sub>	M	F	t <sub>D</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM)	Q <sub>E</sub> (PM)	RFC
A	1.02	7.39	0.10	2240	1.45	0.76	2163	1183	0.09 0.16
B	1.06	12.21	0.10	3700	1.45	1.05	3797	3775	0.43 0.36
C	1.09	8.85	0.10	2683	1.45	0.85	1957	1913	0.06 0.72

Critical Arm: B C  
 RFC: 0.43 0.72

- In accordance with TPDM V2.4

## APPENDIX 6A

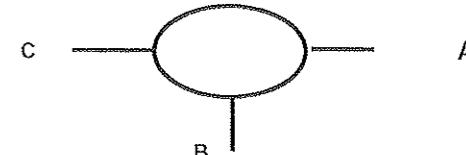
### Simplified Roundabout Capacity Calculation

**ATKINS**

Job Title:	Ocean Park Hotel Development Feasibility Study
Junction:	J3 - Ocean Park Road / Heung Yip Road Roundabout
Scheme:	Reference (Redevelopment+ No Hotel+ No SIL)
Design Year:	2022

Job No.:	4074	Date :	1/25/2007
----------	------	--------	-----------

ARM A: Heung Yip Road  
 ARM B: Ocean Park Road (to WCH Rd)  
 ARM C: Ocean Park Road (to Main Entrance)



GEOMETRY							
ARM	v	e	L	r	D	Phi	S
A	5.50	8.00	25	12	37	15	0.16
B	11.00	12.50	20	33	37	17.5	0.12
C	5.50	11.00	27.5	100	37	15	0.32

AM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	90	110	118	210
B	893	40	648	130	1581
C	10	68	10	943	88

Flow in pcu/hr

PM FLOWS					
from/to	A	B	C	Circ	Entry
A	10	70	130	1591	210
B	1033	70	261	150	1364
C	30	1511	10	1113	1551

Flow in pcu/hr

CALCULATIONS									
ARM	K	X <sub>2</sub>	M	F	t <sub>D</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM)	Q <sub>E</sub> (PM)	RFC
A	1.02	7.39	0.10	2240	1.45	0.76	2193	1056	0.10 0.20
B	1.06	12.21	0.10	3700	1.45	1.05	3786	3764	0.42 0.36
C	1.09	8.85	0.10	2683	1.45	0.85	2056	1899	0.04 0.82

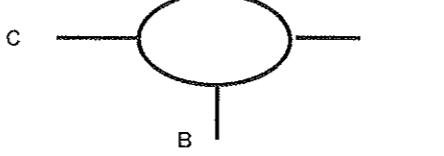
Critical Arm: B C  
 RFC: 0.42 0.82

- In accordance with TPDM V2.4

# APPENDIX 6A

## Simplified Roundabout Capacity Calculation

**ATKINS**

Job Title: Ocean Park Hotel Development Feasibility Study											
Junction: J3 - Ocean Park Road / Heung Yip Road Roundabout				Designed by: JC							
Scheme: Scheme (Redevelopment+ New Hotel+ No SIL)				Checked by: JY							
Design Year: 2022		Job No.: 4074		Date : 1/25/2007							
<p>ARM A: Heung Yip Road          ARM B: Ocean Park Road (to WCH Rd)          ARM C: Ocean Park Road (to Main Entrance)</p>											
											
<b>GEOMETRY</b>											
ARM	v	e	L	r	D	Phi	S				
A	5.50	8.00	25	12	37	15	0.16				
B	11.00	12.50	20	33	37	17.5	0.12				
C	5.50	11.00	27.5	100	37	15	0.32				
<b>AM FLOWS</b>											
from/to	A	B	C	Circ	Entry						
A	10	90	110	157	210						
B	931	40	687	130	1658						
C	10	107	10	981	127						
Flow in pcu/hr											
<b>PM FLOWS</b>											
from/to	A	B	C	Circ	Entry						
A	10	70	130	1637	210						
B	1078	70	307	150	1455						
C	30	1557	10	1158	1597						
Flow in pcu/hr											
<b>RFC</b>											
ARM	K	X <sub>2</sub>	M	F	t <sub>d</sub>	f <sub>c</sub>	Q <sub>E</sub> (AM) Q <sub>E</sub> (PM)				
A	1.02	7.39	0.10	2240	1.45	0.76	2163 1020				
B	1.06	12.21	0.10	3700	1.45	1.05	3786 3764				
C	1.09	8.85	0.10	2683	1.45	0.85	2021 1858				
Critical Arm: B C											
RFC: 0.44 0.86											
- In accordance with TPDM V2.4											

## TRAFFIC SIGNAL CALCULATION SHEET

Junction :	J4 - Ocean Park Road/ Hospital Access Road														
Scheme :	Observed Flows	Designed by:													
YKC	Checked by:	JY													
Traffic Flow (pcu/hr) AM(PM)	34(39)	52(36)													
	52(37)	40(36)													
	407(346)	273(396)													
STAGE DIAGRAM															
1	2	3													
C1 → free flow		A1													
	B1	B1													
C1+B2+A1	IG= 6	G= IG= 6													
C1+B2+H	IG= 6	G= IG= 3													
	G= 8+7	G= 2													
	G= 100	G= 100													
AM Peak Flow Calculations	AM Peak Flow Calculations	PM Peak Flow Calculations													
Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in % g	AM Peak Flow (pcu/hr) f	Proportion turning (%)	Saturation flow (pcu/hr) y value	PM Peak Flow (pcu/hr) f	Proportion turning (%)	Saturation flow (pcu/hr) y value			
Hospital Access Road SB	A1	3	3.50	N	N	10.0	0.0	86	100%	1830	0.047	75	100%	1830	0.041
Ocean Park Road WB	B1	1.2	3.30	N	N	0.0	0.0	273	0%	2085	0.131	396	0%	2085	0.190
	B2	2	3.50	N	N	10.0	0.0	40	100%	1830	0.022	36	100%	1830	0.020
Wong Chuk Hang Road EB	C1	1	3.50	N	N	0.0	0.0	407	0%	2105	0.193	346	0%	2105	0.164
	E	1.2	5GM	+ 7FG	= 12s										
	F	3.1	7GM	+ 7FG	= 14s										
	G	2.3	10GM	+ 8FG	= 18s										
	H	3	8GM	+ 7FG	= 15s										
Capacity Calculation	AM Peak	PM Peak													
Sum of Critical y values - Y	C1+B2+H	C1+B2+A1													
	0.215	0.282													
Lost Time - L	24	15													
Cycle Time - C	100	100													
Practical Capacity - Ypr	0.684	0.765													
Reserve Capacity - RC (%)	218%	182%													

Remark:

Date : 25/Jan/07 Junction : J4 - Ocean Park Road/ Hospital Access Road

ATKINS CHINA LIMITED

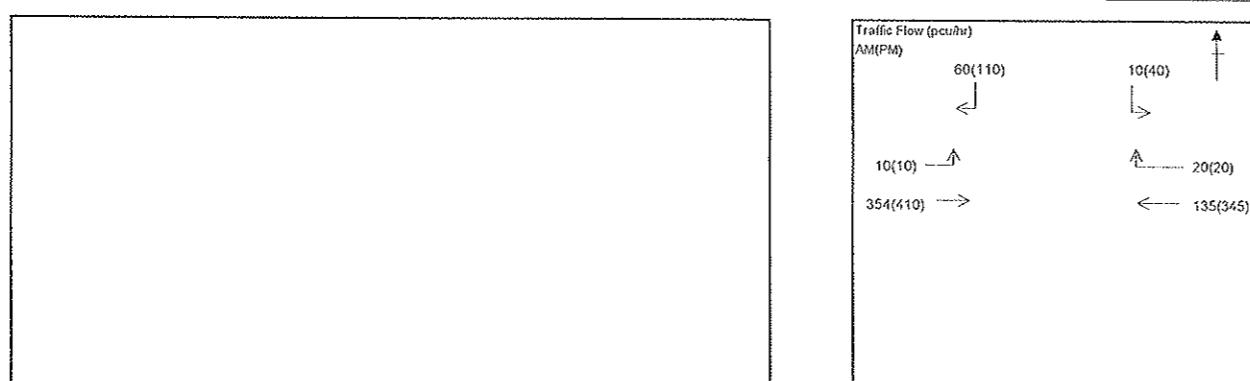
## TRAFFIC SIGNAL CALCULATION SHEET

Junction : J4 - Ocean Park Road/ Hospital Access Road

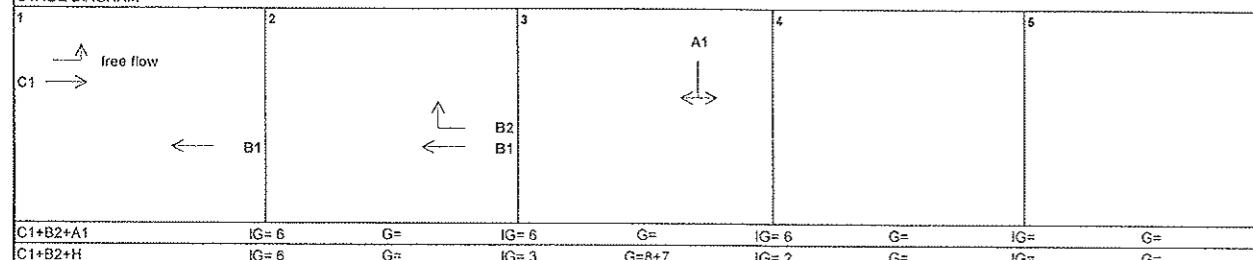
JOB NO. : 4074

Scheme : Reference (Redevelopment + No Hotel + No SII)

Designed by: YKG Checked by: IX



**STAGE DIAGRAM**



## Saturation Flow Calculations

Capacity Calculations

	Current	Optimized	Optimal	Optimal
Sum of Critical Y Values - Y	0.179	0.217	0.206	0.288
Lost Time - L	24	15	24	15
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.684	0.765	0.684	0.765
Reserve Capacity - RC (%)	282%	252%	233%	166%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07 Junction : M - Ocean Park Road/ Hospital Access Road

ATKINS CHINA LIMITED

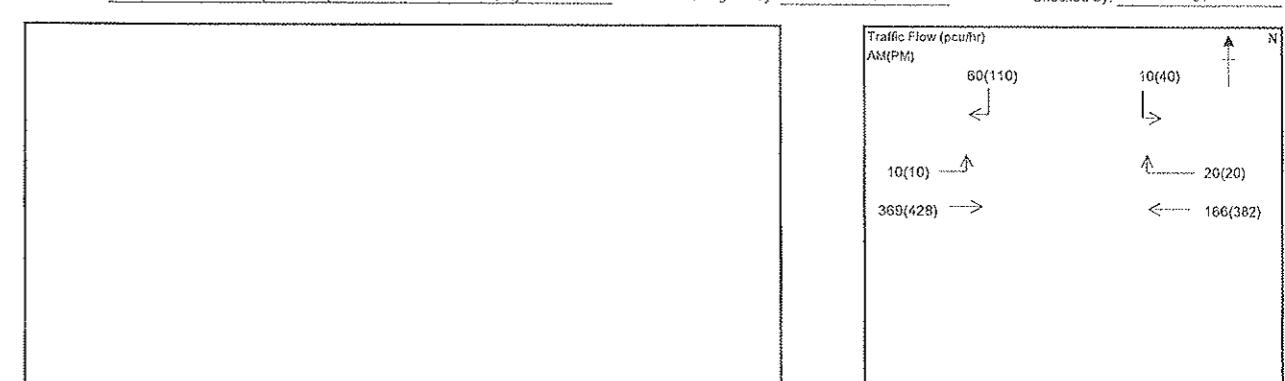
## TRAFFIC SIGNAL CALCULATION SHEET

Junction : M - Ocean Park Road/ Hospital Access Road

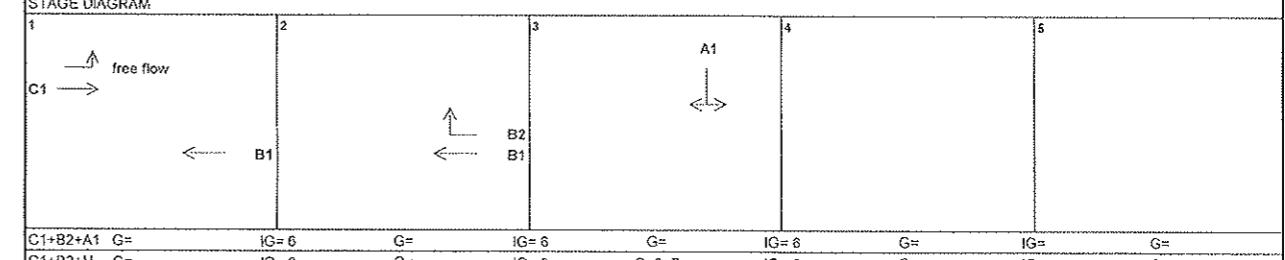
Scheme 1.  $\text{Pd}(\text{PPh}_3)_4$  and  $\text{Al}(\text{OBn})_3$  (1:1, 20 mol % each).

JOB NO : 6974

4074



STAGE DIAGRAM



### Saturation Flow Calculations

Capacity Calculation

Calculation	C1+B2+H	C1+B2+A1	C1+B2+H	C1+B2+A1
Sum of Critical Y Values - Y	0.186	0.224	0.214	0.296
Lost Time - L	24	15	24	15
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.684	0.765	0.684	0.765
Design Capacity - PC (%)	267%	344%	210%	165%

Remarks: Traffic Forecast referenced from Maynard TIA Report (June 2005)

Date : 25/Jan/07 Location : 16, Octave Rue, Batticaloa Hospital Approach Road.

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## APPENDIX 6A

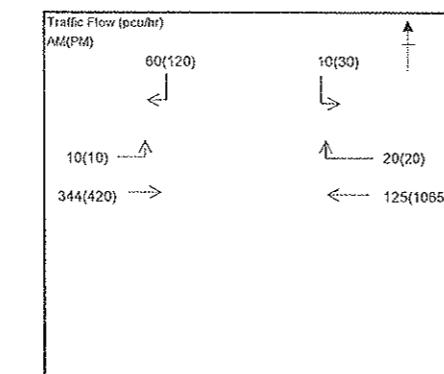
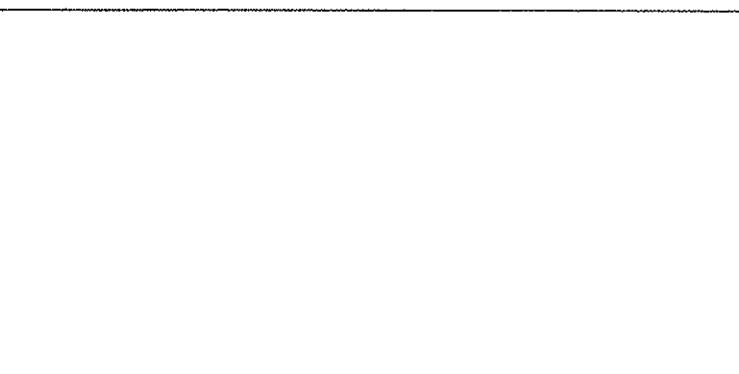
### TRAFFIC SIGNAL CALCULATION SHEET

Junction: J4 - Ocean Park Road/ Hospital Access Road  
Design Year: 2016

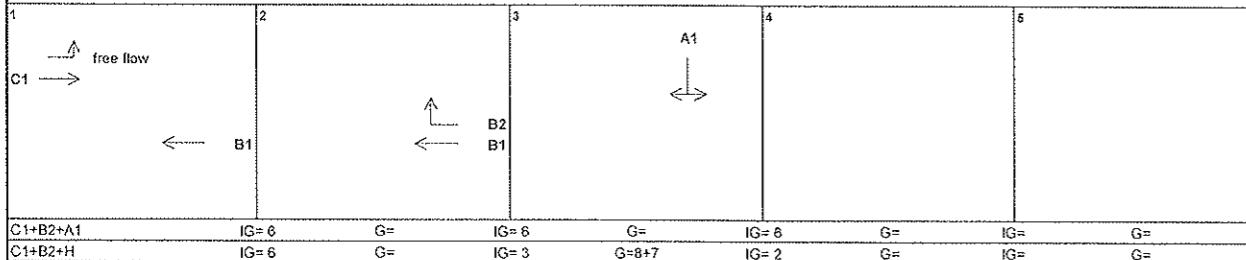
Scheme: Reference (Redevelopment + No Hotel + No S/L)

JOB NO.: 4074

Designed by: YKC      Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Hospital Access Road SB</b>										
A1	3	3.50	N	N	10.0	0.0	70	100%	1830	0.038
							150	100%	1830	0.082
<b>Ocean Park Road WB</b>										
B1	1,2	3.30	N	N	0.0	0.0	125	0%	2085	0.060
B2	2	3.50	N	N	10.0	0.0	20	100%	1830	0.011
							1065	0%	2085	0.511
							20	100%	1830	0.011
<b>Wong Chuk Hang Road EB</b>										
C1	1	3.50	N	N	0.0	0.0	344	0%	2105	0.163
							420	0%	2105	0.200
E	1,2	5GM	+ 7FG	= 12s						
F	3,1	7GM	+ 7FG	= 14s						
G	2,3	10GM	+ 8FG	= 18s						
H	3	8GM	+ 7FG	= 15s						

Capacity Calculation		AM Peak		PM Peak	
		C1+B2+H	C1+B2+A1	B1+A1	C1+B2+A1
Sum of Critical y Values - Y		0.174	0.213	0.533	0.232
Lost Time - L		24	15	10	15
Cycle Time - C		100	100	100	100
Practical Capacity - Ypr		0.684	0.765	0.810	0.765
Reserve Capacity - RC (%)		292%	260%	37%	162%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date: 25/Jan/07

Junction: J4 - Ocean Park Road/ Hospital Access Road

ATKINS CHINA LIMITED

## APPENDIX 6A

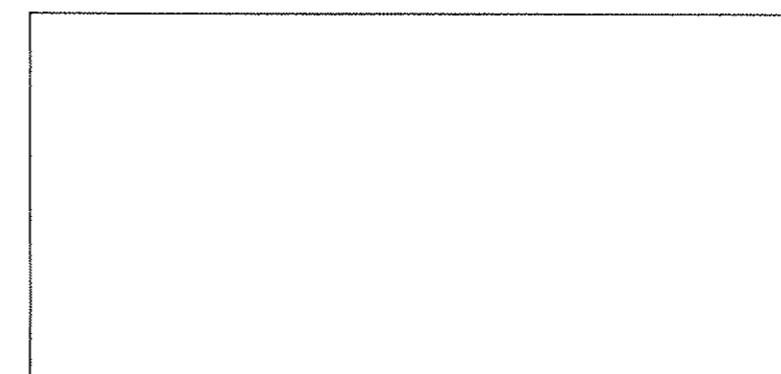
### TRAFFIC SIGNAL CALCULATION SHEET

Junction: J4 - Ocean Park Road/ Hospital Access Road

Design Year: 2016

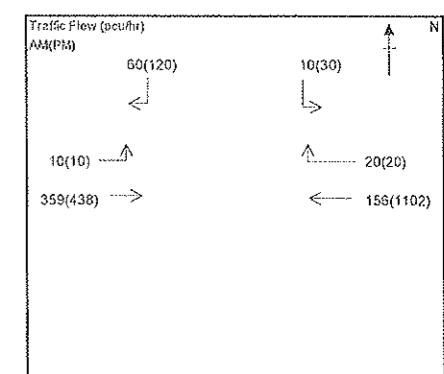
Scheme: Scheme (Redevelopment + New Hotel + No S/L)

Designed by: YKC      Checked by: JY

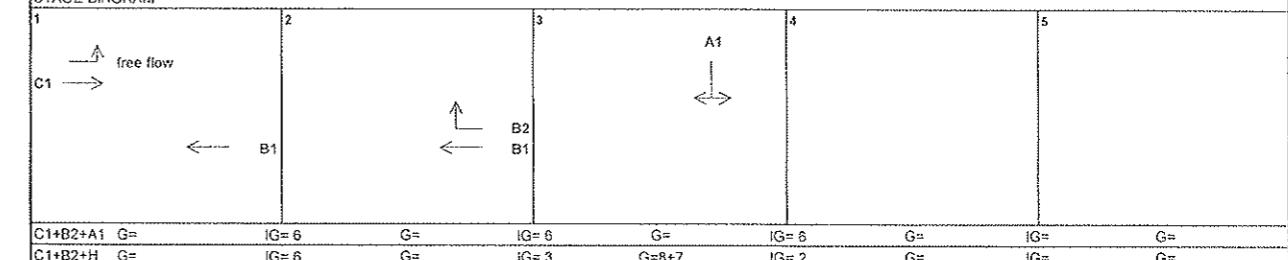


JOB NO.: 4074

Designed by: YKC      Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m) w	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m) r	Gradient in % g	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Hospital Access Road SB</b>										
A1	3	3.50	N	N	10.0	0.0	70	100%	1830	0.038
							150	100%	1830	0.082
<b>Ocean Park Road WB</b>										
B1	1,2	3.30	N	N	0.0	0.0	125	0%	2085	0.075
B2	2	3.50	N	N	10.0	0.0	20	100%	1830	0.011
							1065	0%	2085	0.529
							20	100%	1830	0.011
<b>Wong Chuk Hang Road EB</b>										
C1	1	3.50	N	N	0.0	0.0	359	0%	2105	0.171
							438	0%	2105	0.208
E	1,2	5GM	+ 7FG	= 12s						
F	3,1	7GM	+ 7FG	= 14s						
G	2,3	10GM	+ 8FG	= 18s						
H	3	8GM	+ 7FG	= 15s						

Capacity Calculation		AM Peak		PM Peak	
		C1+B2+H	C1+B2+A1	B1+A1	C1+B2+A1
Sum of Critical y Values - Y		0.181	0.220	0.610	0.301
Lost Time - L		24	15	10	15
Cycle Time - C		100	100	100	100
Practical Capacity - Ypr		0.684	0.765	0.810	0.765
Reserve Capacity - RC (%)		277%	248%	33%	154%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date: 25/Jan/07

Junction: J4 - Ocean Park Road/ Hospital Access Road

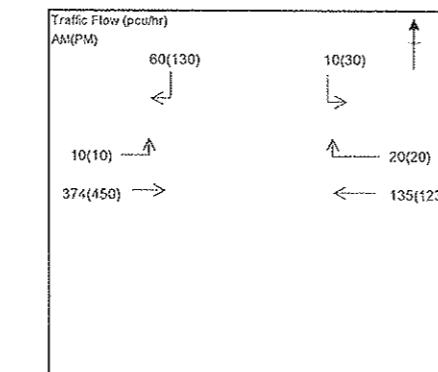
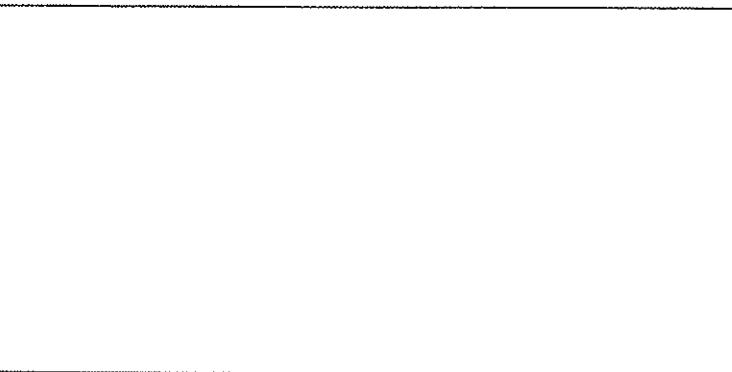
ATKINS CHINA LIMITED

## APPENDIX 6A

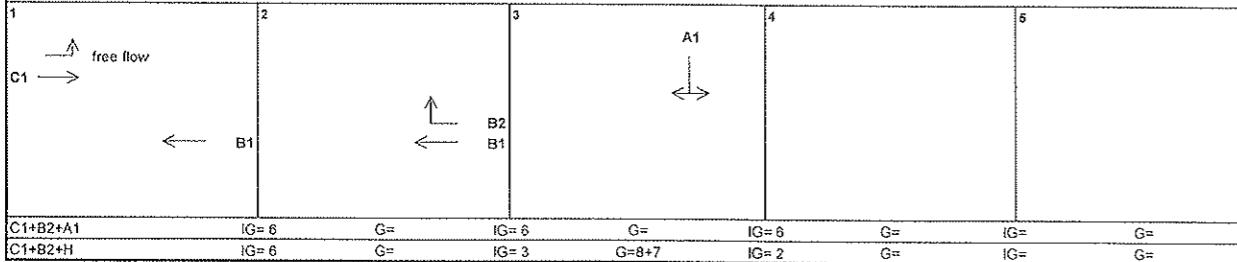
### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J4 - Ocean Park Road/ Hospital Access Road  
 Design Year: 2022

Scheme : Reference (Redevelopment + No Hotel + No S/L) Designed by: YKC Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%)	Saturation flow (pcu/hr)	y value
<b>Hospital Access Road SB</b>										
A1	3	3.50	N	N	10.0	0.0	70	100%	1830	0.038
							160	100%	1830	0.087
<b>Ocean Park Road WB</b>										
B1	1,2	3.30	N	N	0.0	0.0	135	0%	2085	0.065
B2	2	3.50	N	N	10.0	0.0	20	100%	1830	0.011
							1235	0%	2085	0.692
							20	100%	1830	0.011
<b>Wong Chuk Hang Road EB</b>										
C1	1	3.50	N	N	0.0	0.0	374	0%	2105	0.178
							450	0%	2105	0.214
E	1,2	5GM	+ 7FG	= 12s						
F	3,1	7GM	+ 7FG	= 14s						
G	2,3	10GM	+ 8FG	= 18s						
H	3	8GM	+ 7FG	= 15s						

Capacity Calculation	AM Peak		PM Peak	
	C1+B2+H	C1+B2+A1	B1+A1	C1+B2+A1
Sum of Critical y Values - Y	0.189	0.227	0.680	0.312
Lost Time - L	24	15	10	15
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.684	0.765	0.810	0.765
Reserve Capacity - RC (%)	283%	237%	13%	145%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07 Junction : J4 - Ocean Park Road/ Hospital Access Road

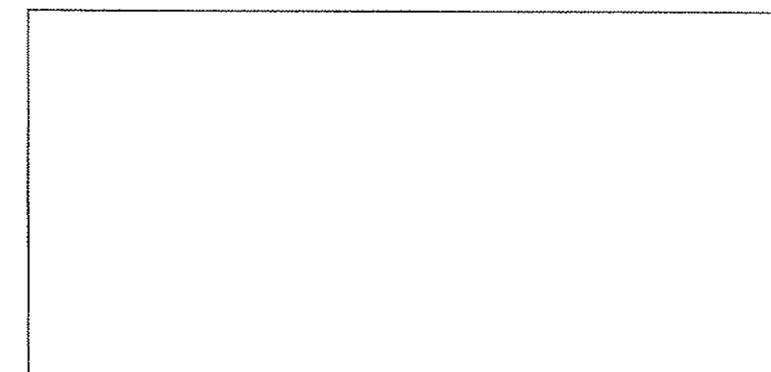
ATKINS CHINA LIMITED

## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J4 - Ocean Park Road/ Hospital Access Road

Scheme : Schema (Redevelopment + New Hotel + No S/L) Designed by: YKC Checked by: JY

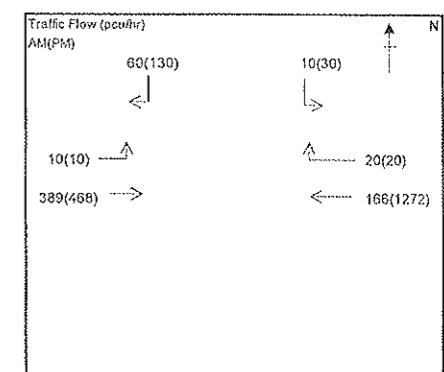


JOB NO. : 4074

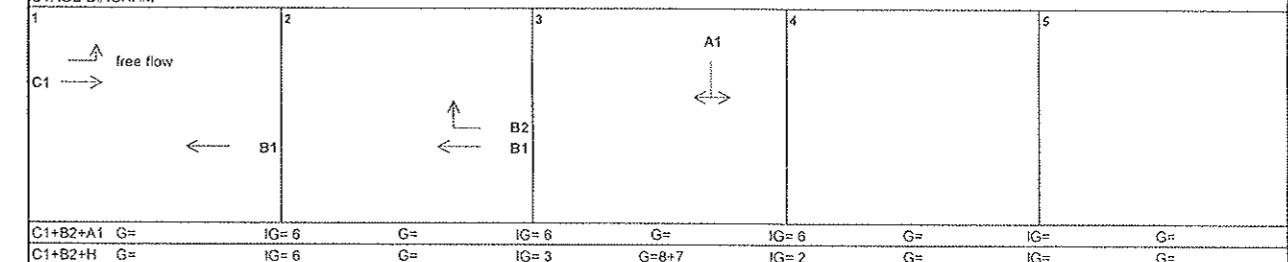
Design Year: 2022

YKC

Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%)	Saturation flow (pcu/hr)	y value
<b>Hospital Access Road SB</b>										
A1	3	3.50	N	N	10.0	0.0	70	100%	1830	0.038
							160	100%	1830	0.087
<b>Ocean Park Road WB</b>										
B1	1,2	3.30	N	N	0.0	0.0	135	0%	2085	0.080
B2	2	3.50	N	N	10.0	0.0	20	100%	1830	0.011
							1272	0%	2085	0.610
							20	100%	1830	0.011
<b>Wong Chuk Hang Road EB</b>										
C1	1	3.50	N	N	0.0	0.0	389	0%	2105	0.185
							468	0%	2105	0.222
E	1,2	5GM	+ 7FG	= 12s						
F	3,1	7GM	+ 7FG	= 14s						
G	2,3	10GM	+ 8FG	= 18s						
H	3	8GM	+ 7FG	= 15s						

Capacity Calculation	AM Peak		PM Peak	
	C1+B2+H	C1+B2+A1	B1+A1	C1+B2+A1
Sum of Critical y Values - Y	0.189	0.234	0.697	0.321
Lost Time - L	24	15	10	15
Cycle Time - C	100	100	100	100
Practical Capacity - Ypr	0.684	0.765	0.810	0.765
Reserve Capacity - RC (%)	249%	227%	16%	139%

Remark: Traffic Forecast referenced from Maunsell TIA Report (June 2005)

Date : 25/Jan/07 Junction : J4 - Ocean Park Road/ Hospital Access Road

ATKINS CHINA LIMITED

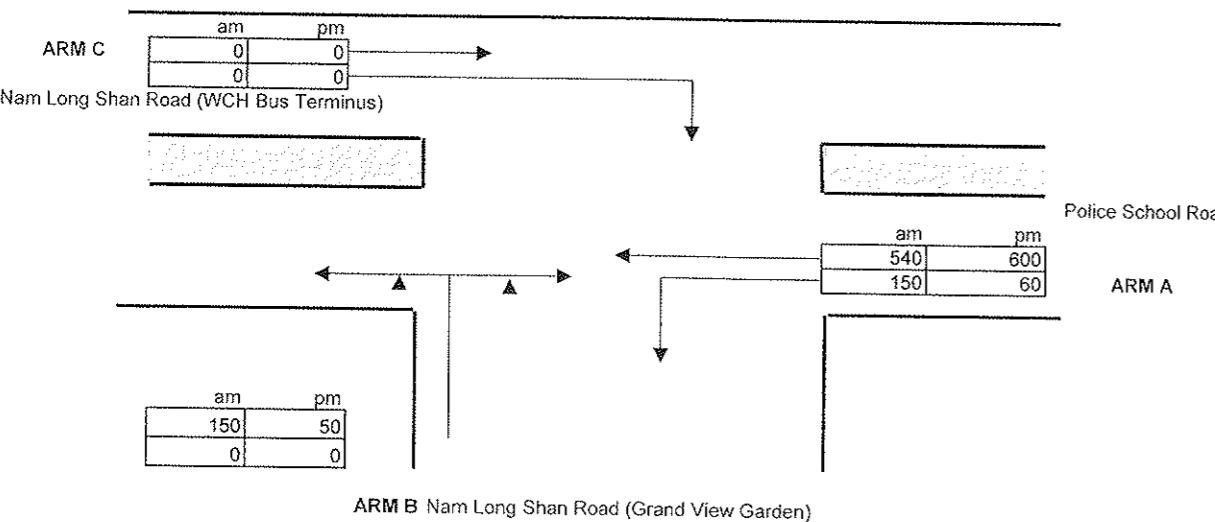
## APPENDIX 6A

### Simplified Priority Junction Capacity Calculation

**ATKINS**

(Single Lane Minor Arm B)

Job Title:	Ocean Park Hotel Development Feasibility Study	
Junction:	J6 - Nam Long Shan Road / Police School Road	Designed by: JC
Scheme:	Observed Flows	Checked by: JY
Design Year:	2006	Job No.: 4074
ARM A:	Police School Road	Date : 1/25/2007
ARM B:	Nam Long Shan Road (Grand View Garden)	
ARM C:	Nam Long Shan Road (WCH Bus Terminus)	



#### GEOMETRY

Major road width	W	14.60	Lane widths	w(b-a)	0.00
Central Reserve width	Wcr	0.00	w(b-c)	3.50	
			w(c-b)	0.00	
Visibilities	Vr(b-a)	80	Calculated	D	0.63
	Vi(b-a)	50		E	0.95
	Vr(b-c)	80		F	0.63
	Vr(c-b)	80		Y	0.50

#### ANALYSIS

TRAFFIC FLOWS		AM PEAK		PM PEAK	
		q(c-a)	q(c-b)	q(a-b)	q(a-c)
		0	0	150	60
				540	600
				0	0
				150	50
				1.00	1.00
CAPACITIES		329		326	
		Q(b-a)	Q(b-c)	Q(c-b)	Q(b-ac)
		329	605	601	605
RFC's		0.000		0.000	
		c-b	b-ac	0.248	0.083
RFC		0.25		0.08	

Where Vi and Vr are visibility distances to the left or right of the respective streams

$$D = (1+0.094(w(b-a)-3.65))(1+0.0009(Vr(b-a)-120))(1+0.0006(Vi(b-a)-150))$$

$$E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))$$

$$F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))$$

$$Y = 1-0.0345W$$

$$f = \text{proportion of minor traffic turning left}$$

$$Q(b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)$$

Capacity of combined streams

- In accordance with TPDM V2.4

T.P.D.M.V.2.4  
Appendix 1

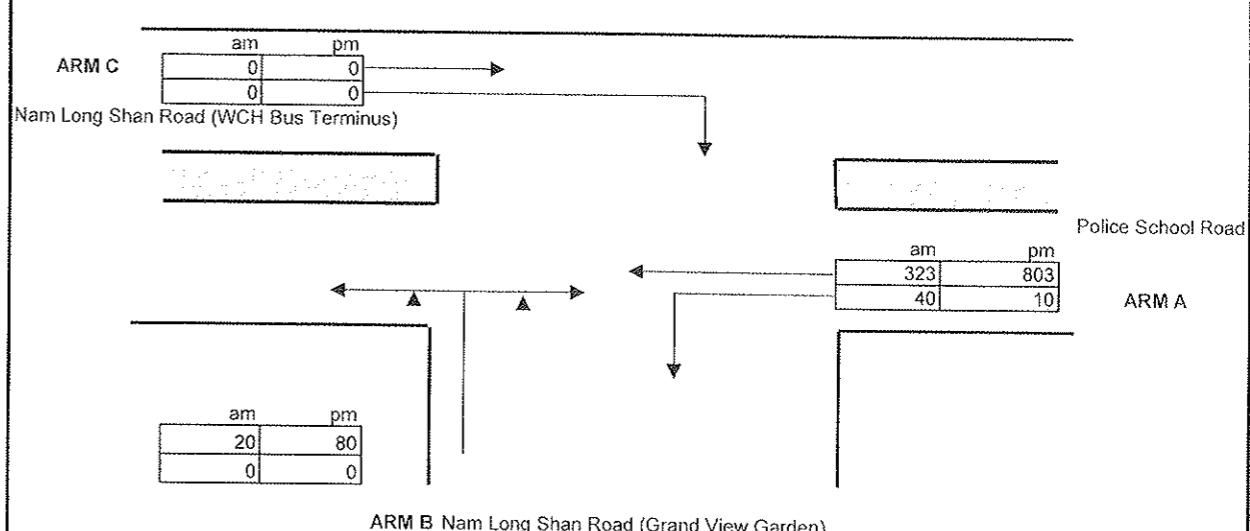
## APPENDIX 6A

### Simplified Priority Junction Capacity Calculation

**ATKINS**

(Single Lane Minor Arm B)

Job Title:	Ocean Park Hotel Development Feasibility Study	
Junction:	J6 - Nam Long Shan Road / Police School Road	Designed by: JC
Scheme:	Reference (Redevelopment + No Hotel + No SIL)	Checked by: JY
Design Year:	2011	Job No.: 4074
ARM A:	Police School Road	Date : 1/25/2007
ARM B:	Nam Long Shan Road (Grand View Garden)	
ARM C:	Nam Long Shan Road (WCH Bus Terminus)	



#### GEOMETRY

Major road width	W	14.60	Lane widths	w(b-a)	0.00
Central Reserve width	Wcr	0.00	w(b-c)	3.50	
			w(c-b)	0.00	
Visibilities	Vr(b-a)	80	Calculated	D	0.63
	Vi(b-a)	50		E	0.95
	Vr(b-c)	80		F	0.63
	Vr(c-b)	80		Y	0.50

#### ANALYSIS

TRAFFIC FLOWS		AM PEAK		PM PEAK	
		q(c-a)	q(c-b)	q(a-b)	q(a-c)
		0	0	40	10
				323	803
				0	0
				20	80
				1.00	1.00
CAPACITIES		323		359	
		Q(b-a)	Q(b-c)	Q(c-b)	Q(b-ac)
		323	650	430	650
RFC's		0.000		0.000	
		c-b	b-ac	0.031	0.140
RFC		0.25	0.08		

Where Vi and Vr are visibility distances to the left or right of the respective streams

$$D = (1+0.094(w(b-a)-3.65))(1+0.0009(Vr(b-a)-120))(1+0.0006(Vi(b-a)-150))$$

$$E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))$$

$$F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))$$

$$Y = 1-0.0345W$$

$$f = \text{proportion of minor traffic turning left}$$

$$Q(b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)$$

Capacity of combined streams

T.P.D.M.V.2.4  
Appendix 1

- In accordance with TPDM V2.4

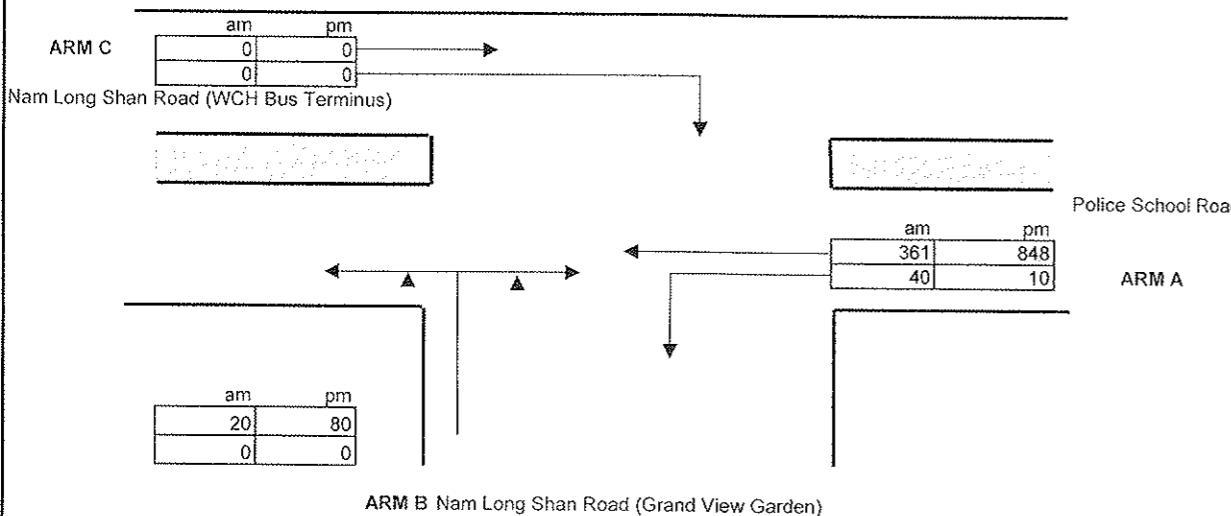
## APPENDIX 6A

### Simplified Priority Junction Capacity Calculation

**ATKINS**

(Single Lane Minor Arm B)

Job Title:	Ocean Park Hotel Development Feasibility Study	
Junction:	J6 - Nam Long Shan Road / Police School Road	Designed by: JC
Scheme:	Scheme (Redevelopment + New Hotel + No SIL)	Checked by: JY
Design Year:	2011	Job No.: 4074
Date :	1/25/2007	
ARM A:	Police School Road	
ARM B:	Nam Long Shan Road (Grand View Garden)	
ARM C:	Nam Long Shan Road (WCH Bus Terminus)	



#### GEOMETRY

Major road width	W	14.60	Lane widths	w(b-a)	0.00
Central Reserve width	Wcr	0.00		w(b-c)	3.50
				w(c-b)	0.00
Visibilities	Vr(b-a)	80	Calculated	D	0.63
	Vl(b-a)	50		E	0.95
	Vr(b-c)	80		F	0.63
	Vr(c-b)	80		Y	0.50

#### ANALYSIS

TRAFFIC FLOWS		AM PEAK		PM PEAK	
		q(c-a)	0	q(c-b)	0
	q(c-b)	0	0	q(a-b)	40
	q(a-b)	40	10	q(a-c)	361
	q(a-c)	361	848	q(b-a)	0
	q(b-a)	0	0	q(b-c)	20
	q(b-c)	20	80	f	1.00
	f	1.00	1.00		
CAPACITIES		355		300	
		Q(b-a)	355	Q(b-c)	643
	Q(b-c)	643	562	Q(c-b)	426
	Q(c-b)	426	374	Q(b-ac)	643
	Q(b-ac)	643	562		
RFC's		0.000		0.000	
		c-b	0.000	b-ac	0.031
RFC		0.03		0.14	

Where Vl and Vr are visibility distances to the left or right of the respective streams

$$D = (1+0.094(w(b-a)-3.65))(1+0.0009(Vr(b-a)-120))(1+0.0006(Vl(b-a)-150))$$

$$E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))$$

$$F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))$$

$$Y = 1-0.0345W$$

f = proportion of minor traffic turning left

$$Q(b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)$$

Capacity of combined streams

T.P.D.M.V.2.4  
Appendix 1

- In accordance with TPDM V2.4

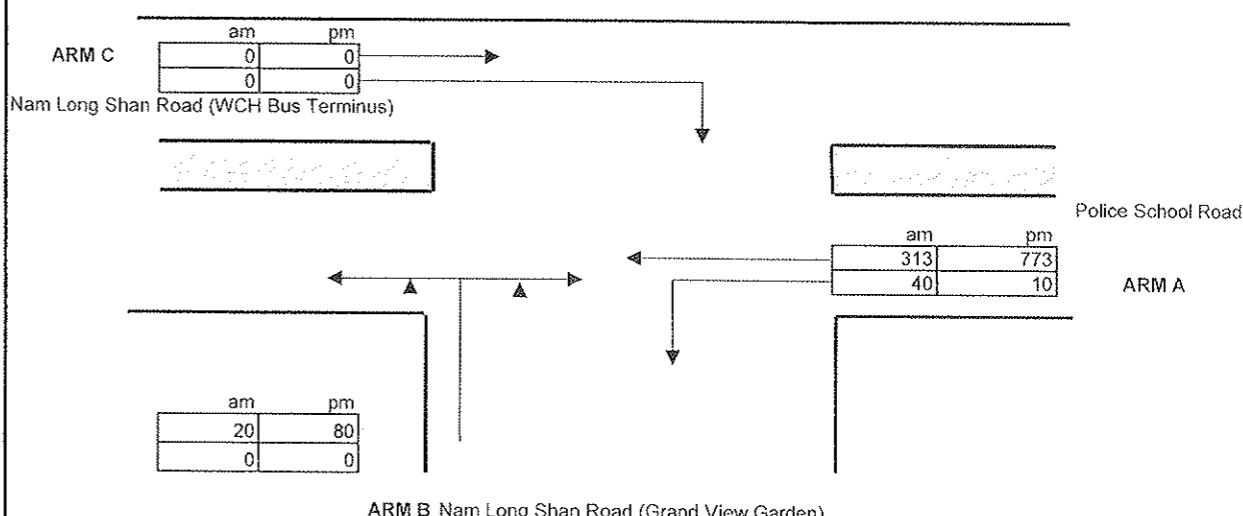
## APPENDIX 6A

### Simplified Priority Junction Capacity Calculation

**ATKINS**

(Single Lane Minor Arm B)

Job Title:	Ocean Park Hotel Development Feasibility Study	
Junction:	J6 - Nam Long Shan Road / Police School Road	Designed by: JC
Scheme:	Reference (Redevelopment + No Hotel + No SIL)	Checked by: JY
Design Year:	2016	Job No.: 4074
Date :	1/25/2007	
ARM A:	Police School Road	
ARM B:	Nam Long Shan Road (Grand View Garden)	
ARM C:	Nam Long Shan Road (WCH Bus Terminus)	



#### GEOMETRY

Major road width	W	14.60	Lane widths	w(b-a)	0.00
Central Reserve width	Wcr	0.00		w(b-c)	3.50
				w(c-b)	0.00
Visibilities	Vr(b-a)	80	Calculated	D	0.63
	Vl(b-a)	50		E	0.95
	Vr(b-c)	80		F	0.63
	Vr(c-b)	80		Y	0.50

#### ANALYSIS

TRAFFIC FLOWS		AM PEAK		PM PEAK	
		q(c-a)	0	q(c-b)	0
	q(c-b)	0	0	q(a-b)	40
	q(a-b)	40	10	q(a-c)	313
	q(a-c)	313	773	q(b-a)	0
	q(b-a)	0	0	q(b-c)	20
	q(b-c)	20	80	f	1.00
	f	1.00	1.00		
CAPACITIES		360		309	
		Q(b-a)	360	Q(b-c)	652
	Q(b-c)	652	575	Q(c-b)	431
	Q(c-b)	431	382	Q(b-ac)	652
	Q(b-ac)	652	575		
RFC's		0.000		0.000	
		c-b	0.000	b-ac	0.031
RFC		0.03		0.14	

Where Vl and Vr are visibility distances to the left or right of the respective streams

$$D = (1+0.094(w(b-a)-3.65))(1+0.0009(Vr(b-a)-120))(1+0.0006(Vl(b-a)-150))$$

$$E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))$$

$$F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))$$

$$Y = 1-0.0345W$$

f = proportion of minor traffic turning left

$$Q(b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)$$

Capacity of combined streams

T.P.D.M.V.2.4  
Appendix 1

- In accordance with TPDM V2.4



## APPENDIX 6A

## Simplified Priority Junction Capacity Calculation

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(Single Lane Minor Arm B)

Job Title:	Ocean Park Hotel Development Feasibility Study				
Junction:	J6 - Nam Long Shan Road / Police School Road	Designed by:	JC		
Scheme:	Scheme (Redevelopment + New Hotel + No SIL)		Checked by:	JY	
Design Year:	2022	Job No.:	4074	Date :	1/25/2007
ARM A:	Police School Road				
ARM B:	Nam Long Shan Road (Grand View Garden)				
ARM C:	Nam Long Shan Road (WCH Bus Terminus)				

**ARM C**

am	pm
0	0
0	0

Nam Long Shan Road (WCH Bus Terminus)

**ARM A**

am	pm
371	868
40	10

**ARM B** Nam Long Shan Road (Grand View Garden)

am	pm
20	80
0	0

**GEOMETRY**

Major road width	W	14.60	Lane widths	w(b-a)	0.00
Central Reserve width	Wcr	0.00	w(b-c)	3.50	
			w(c-b)	0.00	

**Visibilities**

Vr(b-a)	80	Calculated	D	0.63
Vl(b-a)	50		E	0.95
Vr(b-c)	80		F	0.63
Vr(c-b)	80		Y	0.50

**ANALYSIS**

	AM PEAK	PM PEAK
TRAFFIC FLOWS	q(c-a) q(c-b) q(a-b) q(a-c) q(b-a) q(b-c) f	0 0 40 371 0 20 1.00
CAPACITIES	Q(b-a) Q(b-c) Q(c-b) Q(b-ac)	354 642 425 642
RFC's	c-b b-ac	0.000 0.031
RFC		0.03 0.14

Where Vl and Vr are visibility distances to the left or right of the respective streams

$D = (1+0.094(w(b-a)-3.65))(1+0.0009(Vr(b-a)-120))(1+0.0006(Vl(b-a)-150))$

$E = (1+0.094(w(b-c)-3.65))(1+0.0009(Vr(b-c)-120))$

$F = (1+0.094(w(c-b)-3.65))(1+0.0009(Vr(c-b)-120))$

$Y = 1-0.0345W$

f = proportion of minor traffic turning left

$Q(b-ac) = Q(b-c)*Q(b-a)/(1-f)*Q(b-c)+f*Q(b-a)$

Capacity of combined streams

- In accordance with TPDM V2.4

T.P.D.M.V.2.4  
Appendix 1

## TRAFFIC SIGNAL CALCULATION SHEET

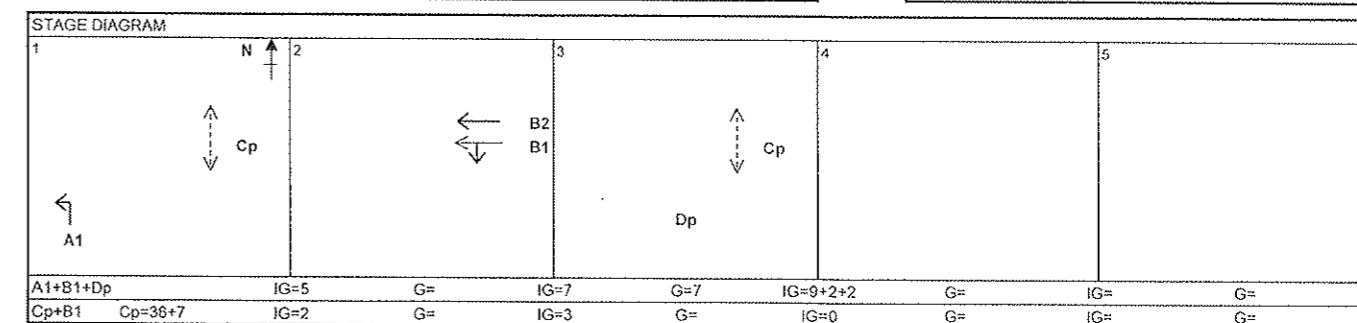
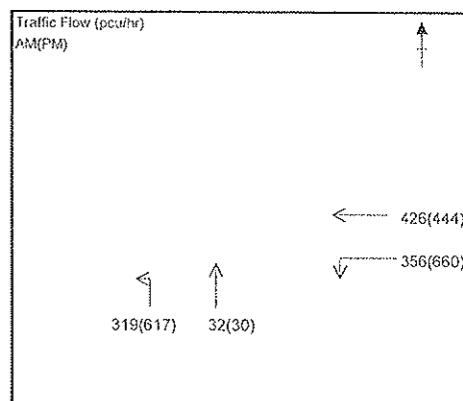
Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

Scheme : Observed Flows

Design Year: 2006

Designed by: YKC

Checked by: JY



## Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
Shum Wan Road	A1	3.75	Y	N	17.5	0.0	351	91%	1846	0.190
Nam Long Shan Road	B1	3.60	Y	N	20.0	0.0	370	96%	1842	0.201
	B2	4.40	Y	N	0.0	0.0	412	0%	2055	0.201
	Cp	1.3					36GM+7FG=43			
	Dp	3					7GM+5FG=16			

Capacity Calculation	AM Peak		PM Peak	
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp
Sum of Critical y Values - Y	0.201	0.391	0.359	0.711
Lost Time - L	47	30	47	30
Cycle Time - C	90	90	90	90
Practical Capacity - Ypr	0.430	0.600	0.430	0.600
Reserve Capacity - RC (%)	114%	54%	20%	-16%

Remark:

Date : 25-Jan-07 Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

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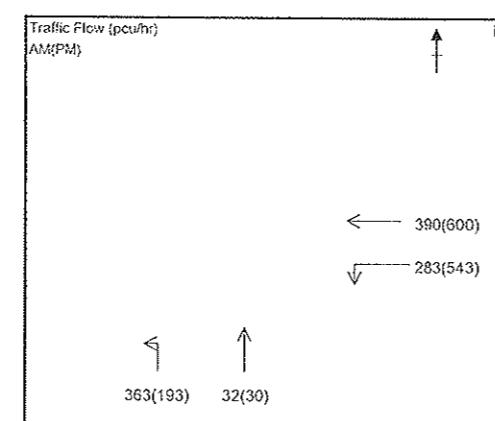
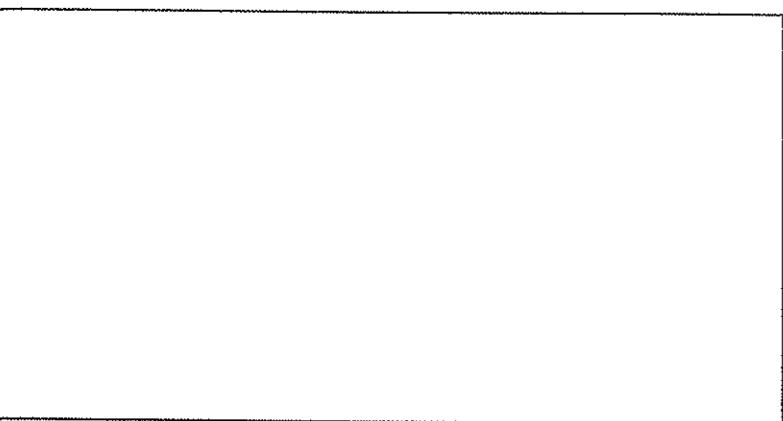
## APPENDIX 6A

### TRAFFIC SIGNAL CALCULATION SHEET

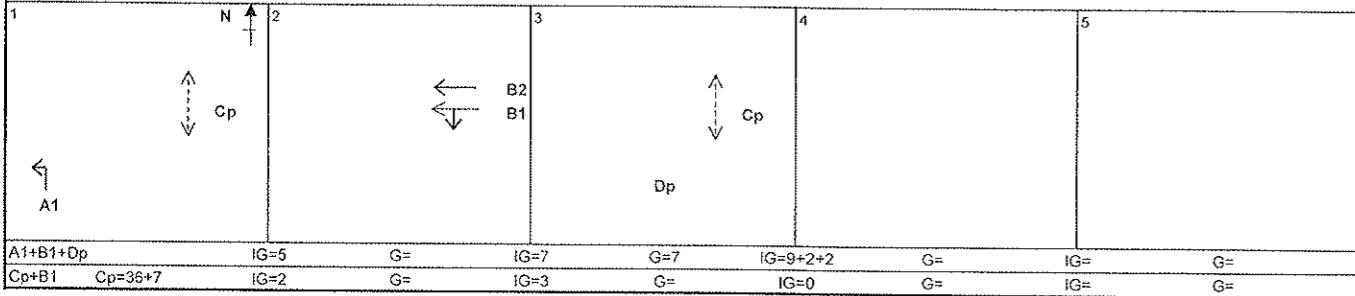
Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

Scheme : Reference (Redevelopment + No Hotel + No SIL)

Designed by: YKC      Checked by: JY



### STAGE DIAGRAM



### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%)	Saturation flow (pcu/hr)	y value
<b>Shum Wan Road</b>										
A1	1	3.75	Y	N	17.5	0.0	395	0%	1990	0.198
B1	2	3.60	Y	N	20.0	0.0	319	89%	1852	0.172
B2	2	4.40	Y	N	0.0	0.0	354	0%	2055	0.172
Cp	1,3									36GM+7FG=43
Dp	3									7GM+9FG=16

Capacity Calculation	AM Peak		PM Peak	
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp
Sum of Critical y Values - Y	0.172	0.371	0.296	0.417
Lost Time - L	47	30	47	30
Cycle Time - C	90	90	90	90
Practical Capacity - Ypr	0.430	0.600	0.430	0.600
Reserve Capacity - RC (%)	150%	62%	45%	44%

Remark:

Date : 25-Jan-07      Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

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## APPENDIX 6A

JOB NO. : 4074

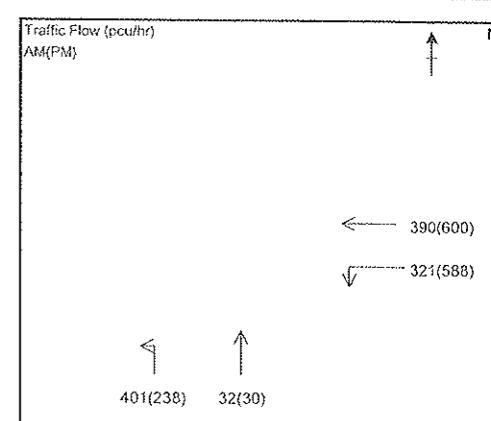
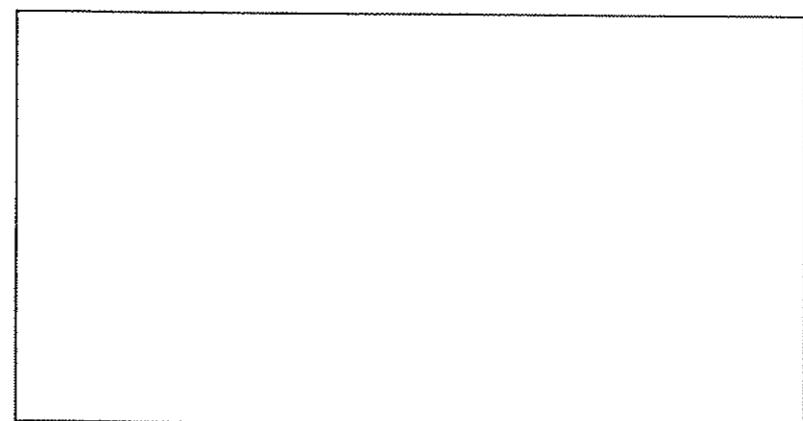
Design Year: 2011

### TRAFFIC SIGNAL CALCULATION SHEET

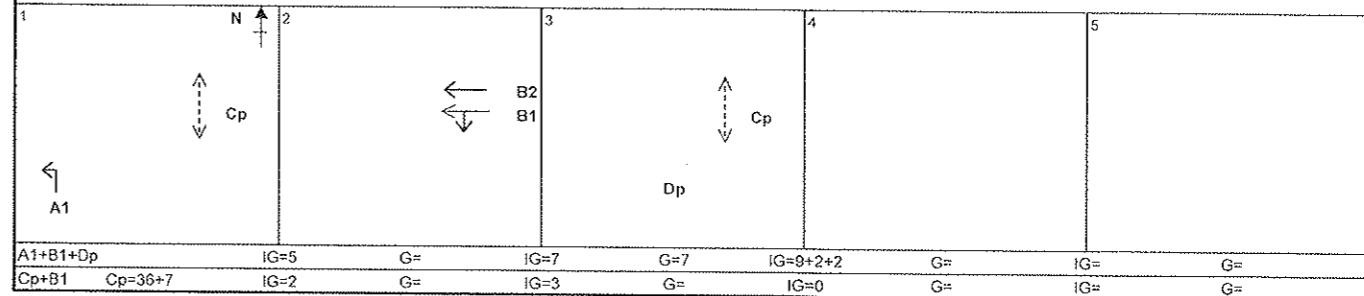
Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

Scheme : Scheme (Redevelopment + New Hotel + No SIL)

Designed by: YKC      Checked by: JY



### STAGE DIAGRAM



### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%)	Saturation flow (pcu/hr)	y value
<b>Shum Wan Road</b>										
A1	1	3.75	Y	N	17.5	0.0	433	0%	1990	0.218
B1	2	3.60	Y	N	20.0	0.0	336	95%	1843	0.182
B2	2	4.40	Y	N	0.0	0.0	375	0%	2055	0.182
Cp	1,3									36GM+7FG=43
Dp	3									7GM+9FG=16

Capacity Calculation	AM Peak		PM Peak	
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp
Sum of Critical y Values - Y	0.182	0.400	0.320	0.466
Lost Time - L	47	30	47	30
Cycle Time - C	90	90	90	90
Practical Capacity - Ypr	0.430	0.600	0.430	0.600
Reserve Capacity - RC (%)	136%	50%	34%	29%

Remark:

Date : 25-Jan-07      Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

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## APPENDIX 6A

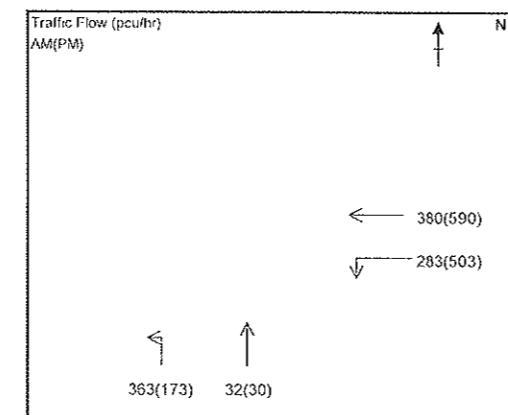
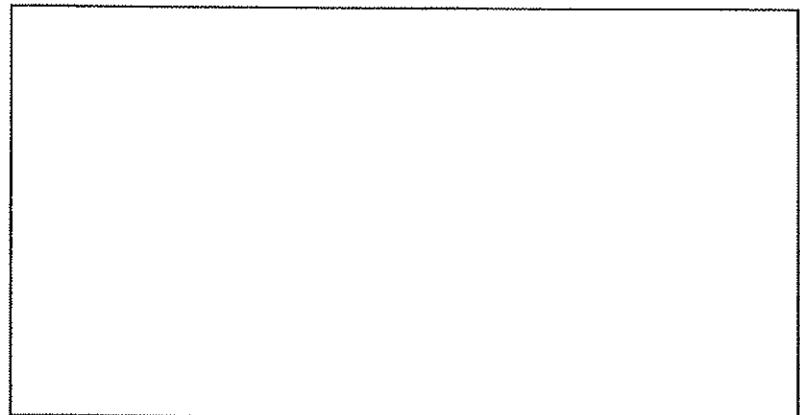
### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

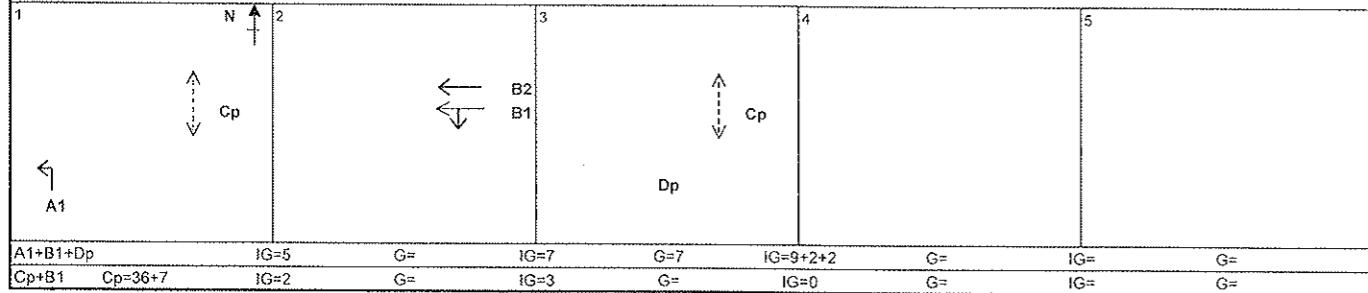
Scheme : Reference (Redevelopment + No Hotel + No SIL)

Design Year: 2016

Designed by: YKC      Checked by: JY



#### STAGE DIAGRAM



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations		
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value	PM Peak Flow (pcu/hr)
<b>Shum Wan Road</b>											
A1	1	3.75	Y	N	17.5	0.0	395	0%	1990	0.198	203
								100%	1833	0.111	
B1	2	3.60	Y	N	20.0	0.0	314	90%	1850	0.170	516
B2	2	4.40	Y	N	0.0	0.0	349	0%	2055	0.170	577
								97%	1841	0.281	
Cp	1,3										
Dp	3										
<b>Nam Long Shan Road</b>											
B1	2	3.60	Y	N	20.0	0.0	314	90%	1850	0.170	516
B2	2	4.40	Y	N	0.0	0.0	349	0%	2055	0.170	577
								97%	1841	0.281	
Cp	1,3										
Dp	3										
<b>36GM+7FG=43 7GM+9FG=16</b>											

Capacity Calculation	AM Peak		PM Peak	
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp
Sum of Critical y Values - Y	0.170	0.368	0.281	0.391
Lost Time - L	47	30	47	30
Cycle Time - C	90	90	90	90
Practical Capacity - Ypr	0.430	0.600	0.430	0.600
Reserve Capacity - RC (%)	153%	63%	53%	53%

Remark:  
Date : 25-Jan-07      Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

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## APPENDIX 6A

JOB NO. : 4074

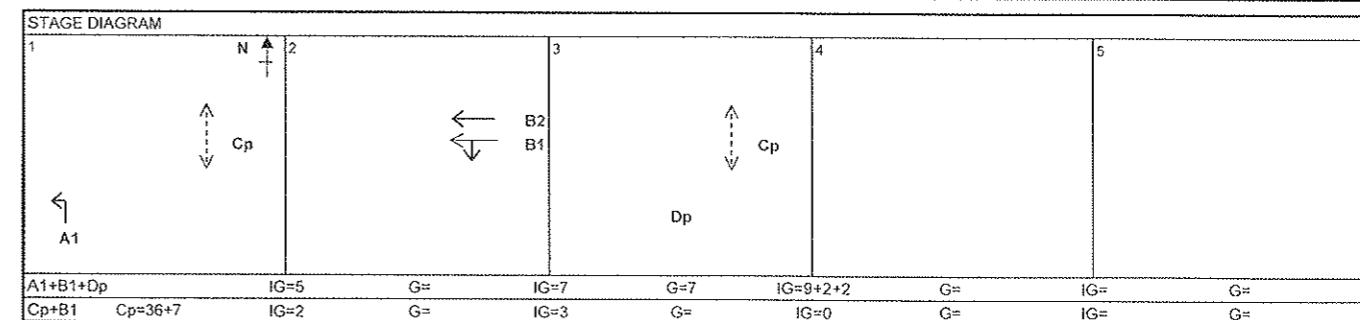
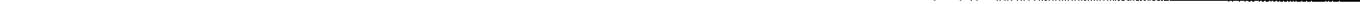
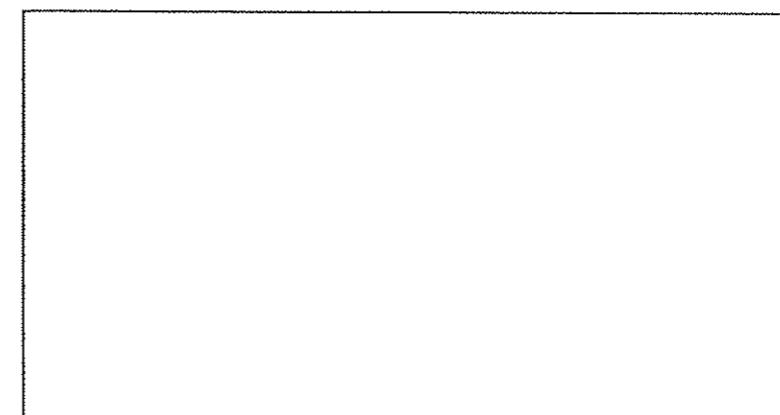
### TRAFFIC SIGNAL CALCULATION SHEET

Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

Scheme : Scheme (Redevelopment + New Hotel + No SIL)

Design Year: 2016

Designed by: YKC      Checked by: JY



#### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations		
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value	PM Peak Flow (pcu/hr)
<b>Shum Wan Road</b>											
A1	1	3.75	Y	N	17.5	0.0	433	0%	1990	0.218	248
								100%	1833	0.135	
Nam Long Shan Road	2	3.60	Y	N	20.0	0.0	331	97%	1841	0.180	548
B2	2	4.40	Y	N	0.0	0.0	370	0%	2055	0.180	590
								100%	1837	0.298	
Cp	1,3										
Dp	3										
<b>36GM+7FG=43 7GM+9FG=16</b>											

Capacity Calculation	AM Peak		PM Peak	
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp
Sum of Critical y Values - Y	0.180	0.398	0.298	0.434
Lost Time - L	47	30	47	30
Cycle Time - C	90	90	90	90
Practical Capacity - Ypr	0.430	0.600	0.430	0.600
Reserve Capacity - RC (%)	139%	51%	44%	38%

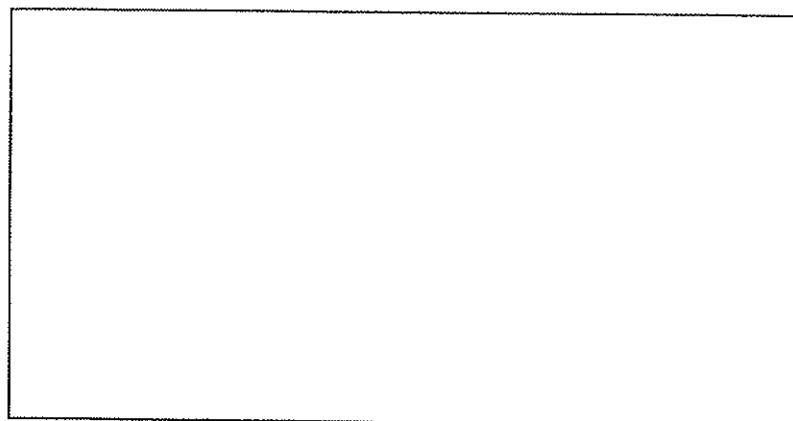
Remark:  
Date : 25-Jan-07      Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

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## TRAFFIC SIGNAL CALCULATION SHEET

Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)  
 Scheme : Reference (Redevelopment + No Hotel + No SIL)

Design Year: 2022  
 Designed by: YKC  
 Checked by: JY

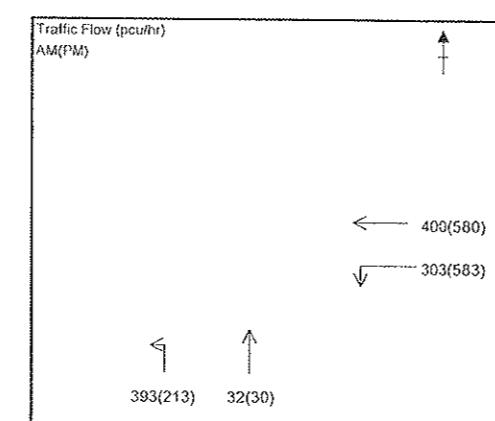


## APPENDIX 6A

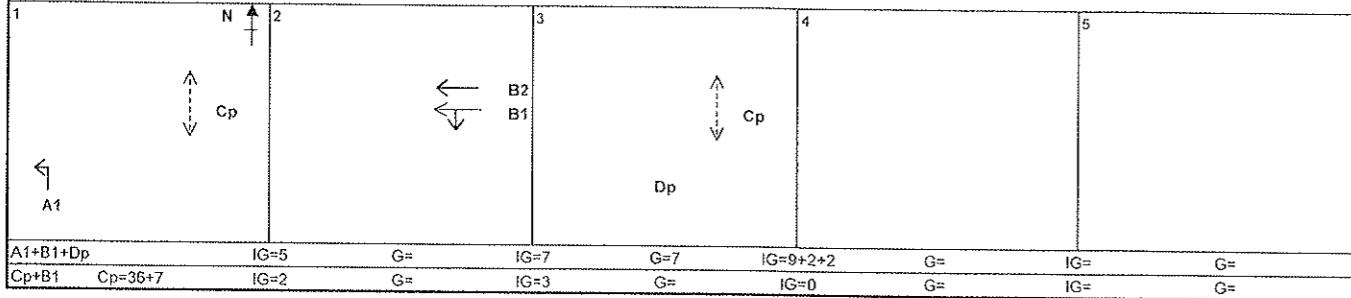
JOB NO. : 4074

Design Year: 2022

Designed by: YKC  
 Checked by: JY



### STAGE DIAGRAM



### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Shum Wan Road</b>										
A1	1	3.75	Y	N	17.5	0.0	425	0%	1990	0.214
B1	2	3.60	Y	N	20.0	0.0	333	91%	1849	0.180
B2	2	4.40	Y	N	0.0	0.0	370	0%	2055	0.180
Cp	1,3									
Dp	3									
36GM+7FG=43 7GM+9FG=16										

### Capacity Calculation

Capacity Calculation	AM Peak		PM Peak		
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp	
	Sum of Critical y Values - Y	0.180	0.394	0.317	0.450
	Lost Time - L	47	30	47	30
Practical Capacity - Ypr	90	90	90	90	
Reserve Capacity - RC (%)	139%	52%	36%	33%	

Remark:

Date : 25-Jan-07 Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

ATKINS CHINA LIMITED

## APPENDIX 6A

JOB NO. : 4074

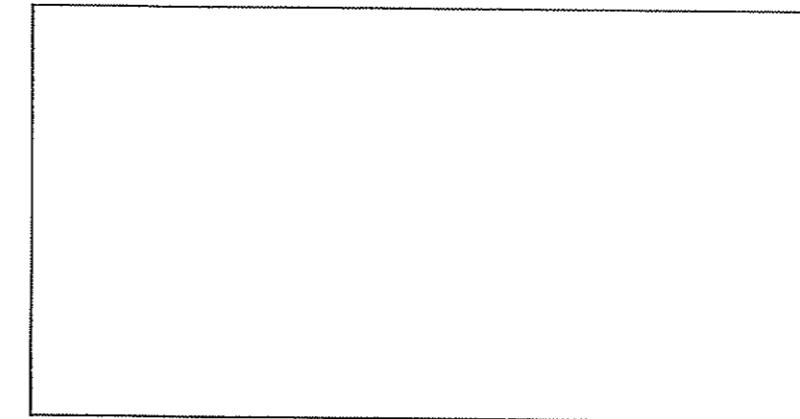
Design Year: 2022  
 Designed by: YKC  
 Checked by: JY

## TRAFFIC SIGNAL CALCULATION SHEET

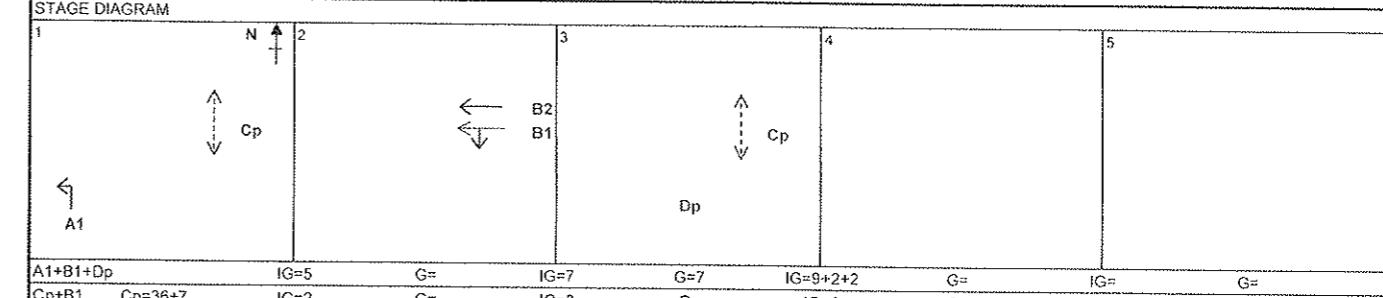
Junction : J7 - Nam Leng Shan Road / Shum Wan Road (W17)

Scheme : Scheme (Redevelopment + New Hotel + No SIL)

Designed by: YKC  
 Checked by: JY



### STAGE DIAGRAM



### Saturation Flow Calculations

Phase	Stage	Width (m)	Nearside? (Y/N)	Opposed? (Y/N)	Radius for turning (m)	Gradient in %	AM Peak Flow Calculations		PM Peak Flow Calculations	
							AM Peak Flow (pcu/hr)	Proportion turning (%) f	Saturation flow (pcu/hr)	y value
<b>Shum Wan Road</b>										
A1	1	3.75	Y	N	17.5	0.0	463	0%	1990	0.233
B1	2	3.60	Y	N	20.0	0.0	350	97%	1841	0.190
B2	2	4.40	Y	N	0.0	0.0	391	0%	2055	0.190
Cp	1,3									
Dp	3									
36GM+7FG=43 7GM+9FG=16										

### Capacity Calculation

Capacity Calculation	AM Peak		PM Peak		
	Cp+B1	A1+B1+Dp	Cp+B1	A1+B1+Dp	
	Sum of Critical y Values - Y	0.190	0.423	0.342	0.499
	Lost Time - L	47	30	47	30
Practical Capacity - Ypr	90	90	90	90	
Reserve Capacity - RC (%)	126%	42%	26%	20%	

Remark:

Date : 25-Jan-07 Junction : J7 - Nam Long Shan Road / Shum Wan Road (W17)

ATKINS CHINA LIMITED